## PENNY LANE SOUTH

102406-9055 CLUFF: THE SOUTH 198 FEET OF THE NORTH HALF OF THE EAST HALF OF THE WEST HALF OF THE NORTHEAST QUARTER OF THE NORTHEAST QUARTER OF SECTION 10, TOWNSHIP 24 NORTH, RANGE 6 EAST, W.M., IN KING COUNTY, EXCEPT THE EAST 30 FEET THEREOF AS CONVEYED TO KING COUNTY FOR 242ND AVENUE SOUTHEAST BY DEED RECORDED UNDER RECORDING NO. 6451729. SITUATE IN THE COUNTY OF KING STATE OF WASHINGTON.

THE NORTH 330 FEET OF THE SOUTH HALF OF THE EAST HALF OF THE WEST HALF OF THE NORTHEAST QUARTER OF THE NORTHEAST QUARTER OF SECTION 10, TOWNSHIP 24 NORTH, RANGE 6 EAST, W.M., IN KING COUNTY, EXCEPT THE EAST 30 FEET THEREOF CONVEYED TO KING COUNTY FOR ROAD BY DEED RECORDED UNDER RECORDING NO. 6448571.

QUARTER OF THE NORTHEAST QUARTER OF SECTION 10, TOWNSHIP 24 NORTH, RANGE 6 EAST, W.M., IN KING

### A NON-EXCLUSIVE EASEMENT FOR INGRESS, EGRESS AND UTILITIES OVER THE SOUTH 30 FEET OF THE EAST HALF OF THE WEST HALF OF THE NORTHEAST QUARTER OF THE NORTHEAST QUARTER OF SAID SECTION 10.

A NON-EXCLUSIVE EASEMENT FOR INGRESS AND EGRESS AS ESTABLISHED UNDER RECORDING NO. 8407170531.

..D.R. STRONG CONSULTING ENGINEERS, INC.

D.R. STRONG CONSULTING ENGINEERS, INC.

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TRAFFIC ENGINEER..

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.... EARTH SOLUTIONS NW, INC.

.11410 NE 124TH ST. #590 .KIRKLAND, WASHINGTÖN 98034

.CONTACT: VINCENT J. GEGLIA

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CONTACT: DALE A. SMITH, P.L.S

CREATIVE LANDSCAPE SOLUTIONS

SEWALL WETLAND CONSULTING, INC.

FALL CITY, WASHINGTON 98024

.CONTACT: MAHER A. JOUDI, P.E. . MAHER.JOUDI@DRSTRONG.COM

SECTION 10, TOWNSHIP 24 NORTH, RANGE 6 EAST, W.M., IN KING COUNTY, WASHINGTON; EXCEPT THE NORTH 330 FEET; AND EXCEPT THE WEST 226 FEET OF THE NORTH 155 FEET OF THE SOUTH 185 FEET; AND EXCEPT THE EAST 30 FEET FOR 242ND STREET SOUTHEAST.

LOT 2 OF KING COUNTY SHORT PLAT NO. 981094, RECORDED UNDER RECORDING NO. 8209160393, RECORDS OF KING COUNTY, WASHINGTON. EXCEPT THE WEST 35 FEET THEREOF. (ALSO KNOWN AS LOT B OF KING COUNTY BOUNDARY LINE ADJUSTMENT NO. L93L0189, RECORDED MARCH 03, 1994 UNDER RECORDING NO. 9403030603, IN THE OFFICIAL RECORDS OF KING COUNTY, WASHINGTON.)

### A NON-EXCLUSIVE EASEMENT FOR INGRESS AND EGRESS OVER THE NORTH 30 FEET OF THE EAST HALF OF THE WEST HALF OF SAID NORTHEAST QUARTER OF THE NORTHEAST QUARTER, AND OVER THE NORTH 30 FEET OF THE WEST HALF OF THE SOUTHEAST QUARTER OF THE NORTHEAST QUARTER OF SECTION 10, TOWNSHIP 24 NORTH, RANGE 6 EAST, W.M., IN KING COUNTY,

SOUTHEASTQUARTER OF THE NORTHEAST QUARTER OF SECTION 10, TOWNSHIP 24 NORTH, RANGE 6 COUNTY FOR ROADWAY PURPOSES UNDER RECORDING NUMBER 7812010976.

QUARTER OF THE SOUTHEAST QUARTER OF THE NORTHEAST QUARTER OF SECTION 10, TOWNSHIP

KING COUNTY PUBLIC WORKS SURVEY BRANCH (KCPWSB) VERTICAL CONTROL POINT NUMBER 2225, AT THE INTERSECTION OF 24TH ST AND 244TH AVE SE, FOUND CONCRETE MONUMENT WITH 3" BRASS DISK, 1.3' BELOW GRADE, IN A MONUMENT CASE, STAMPED WITH THE AGENCY, THE YEAR, THE SECTIONS, THE TOWNSHIP AND THE RANGE. ELEVATION = 403.32 FEET.

### VERTICAL DATUM:

NAVD 88 PER KING COUNTY PUBLIC WORKS SURVEY BRANCH (KCPWSB) VERTICAL CONTROL

### HORIZONTAL DATUM:

### BASIS OF BEARINGS:

N88'10'58"W BETWEEN THE MONUMENTS FOUND IN PLACE AT THE NORTHEAST SECTION CORNER AND THE NORTHQUARTER CORNER SECTION 10-24-6 PER REFERENCE TO

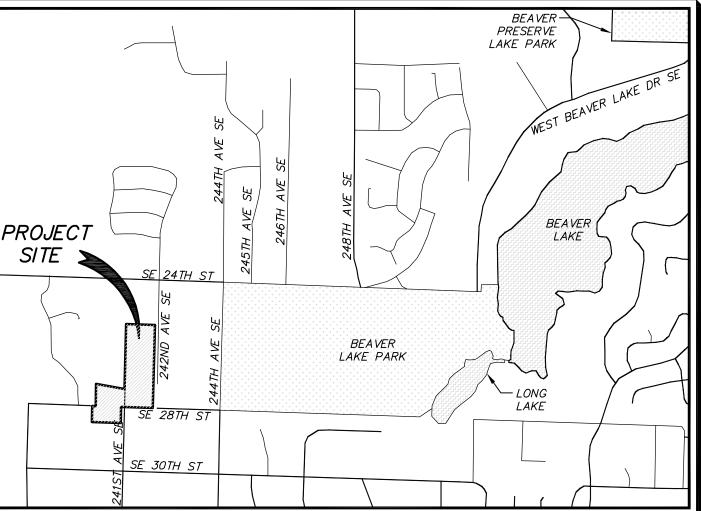
### PROJECT VESTING DATE:

### NOVEMBER 17, 2015

<sup>2</sup> 설심하유민

### INSPECTION NOTIFICATION:

CITY OF SAMMAMISH PUBLIC WORKS DEPARTMENT INSPECTION NOTIFICATION PHONE NUMBER:



VICINITY MAP

C14 14 OF 31

C16 16 OF 31

C17 17 OF 31

C19 19 OF 31

C20 20 OF 31

C22 22 OF 31

C24 24 OF 31

C26 26 OF 31

C27 27 OF 31

C28 28 OF 31

C29 29 OF 31

C30 30 OF 31

CH 1 OF 1

L-1 1 OF 2

1 1 OF 1

R1 1 OF 1

L-2 2 OF 2



ROAD AND STORM DRAINAGE PLAN

242ND AVE SE ROAD AND STORM PROFILE 242ND AVE SE ROAD AND STORM PROFILE

242ND AVE SE ROAD AND STORM PROFILE 241ST AVE SE ROAD AND STORM PROFILE

241ST AVE SE ROAD AND STORM PROFILE

SE 26TH PL ROAD AND STORM PROFILE SE 28TH ST ROAD AND STORM PROFILE

SE 28TH ST ROAD AND STORM PROFILE VAULT CROSS SECTION AND DETAILS

VAULT CROSS SECTION AND DETAILS

CHANNELIZATION AND SIGNAGE PLAN

LANDSCAPE PLAN- TRACTS B AND C

STREET TREE AND REPLACEMENT TREE PLAN

STORM DRAINAGE DETAILS

STORM DRAINAGE DETAILS

RAIN GARDEN DETAILS

FIRE HYDRANT PLAN

STREET LIGHTING PLAN







C1 1 OF 31 EXISTING CONDITIONS TREE RETENTION PLAN TREE RETENTION TABLE C6 6 OF 31 HORIZONTAL CONTROL PLAN C7 7 OF 31 C8 8 OF 31 T.E.S.C. PLAN

## T.E.S.C. NOTES AND DETAILS

C10 10 OF 31 T.E.S.C. DETAILS GRADING PLAN C11 11 OF 31 ADA RAMP DETAILS C12 12 OF 31 C13 13 OF 31 HAMMERHEAD DETAIL

ROAD DETAILS

ROAD DETAILS

ROAD DETAILS

ROAD CROSS-SECTIONS

# C18 18 OF 31

WORK WITHIN EX. R.O.W. AREA: ..... 0.814 ACRES (35,441 S.F.) SINGLE FAMILY DETACHED HOUSING TOTAL PROPOSED IMPERVIOUS: ...... 5.097 ACRES INCLUDING FRONTAGE

. 24106 & 24124 SE 28TH ST; 2525 & 2627

1024069139, 1024069055, 1024069038,

1024069066, 1024069180 & 1024069104

242ND AVE SE SAMMAMISH, WA

.. 8.531 ACRES (371,612 S.F.) GROSS

..1.616 ACRES (70,411 S.F.)

.. 6.915 ACRES (301,201 S.F.)

SURROUNDING LAND USE:.... . SINGLE FAMILY RESIDENTIAL ... SAMMAMISH PLATEAU WATER 425-392-6256 SEWER AND WATER DISTRICT:.....

SCHOOL DISTRICT:... ISSAQUAH NO 411

FIRE DISTRICT:... . EASTSIDE FIRE AND RESCUE 425-313-3200 TELEPHONE SERVICE:... . CENTURY LINK 1-800-366-8201

POWER SOURCE:... . PUGET SOUND ENERGY 1-888-225-5773 CABLE SERVICE:.. .. COMCAST 1-800-266-2278

REC. SPACE REQUIRED ....

PROJECT DESCRIPTION:

PROPOSED DWELLING UNITS:.....

PARCEL NUMBERS ...

EXISTING ZONING:

R.O.W. DEDICATION: ..

PROPOSED USE.....

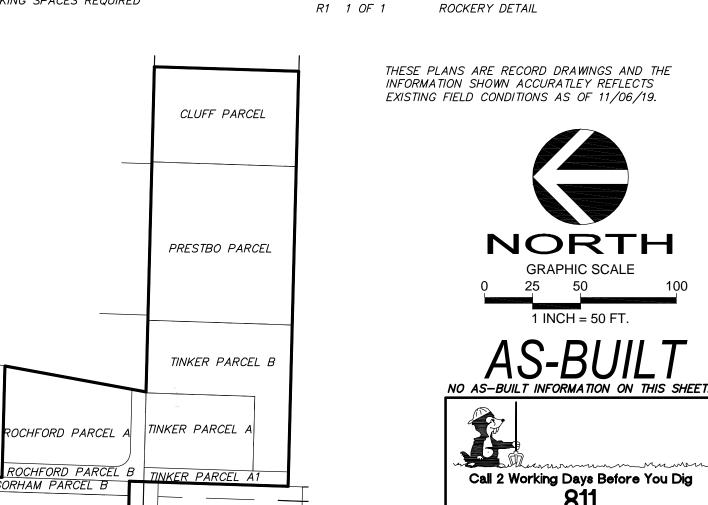
NET SITE AREA: ....

ACREAGE: ...

REC. SPACE PROVIDED ... 23,615 S.F. (TRACT "B") 36,044 S.F. ADDITIONAL PROVIDED IN OPEN SPACE

. 2 PER UNIT, TO BE LOCATED IN GARAGE AND DRIVEWAY PARKING REQUIREMENT:. 56 TOTAL PARKING SPACES REQUIRED

AVERAGE LOT SIZE: ... .. 8,627 S.F.



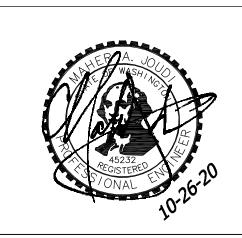
KEY MAP NORTH

GORHAM PARCEL A

Utilities Underground Location Center (ID.MT.ND.OR.WA) SUBDIVISION City of Sammamish Approval Examined and Approved per SMC 20.05 for SDP2017-00575 this\_\_\_\_day of\_\_\_ City Planner Public Works Development Review Engineer

 $R: \2015\0\15065\3\Drawings\As-builts\Plots\AB\_01-3CVR15065.dwg\ 12/17/2019\ 11:38:39\ AM$ COPYRIGHT © 2017, D.R. STRONG CONSULTING ENGINEERS INC.

D.R. STRONG CONSULTING ENGINEERS ENGINEERS PLANNERS SURVEYORS 620 - 7th AVENUE KIRKLAND, WA 98033 O 425.827.3063 F 425.827.2423



APR MAJ MAJ MAJ

7E 73. DA 06. 07. 11.

DRAFTED BY: CEN DESIGNED BY: YLP PROJECT ENGINEER: MAJ

DATE: **02.15.17** PROJECT NO.: **15065** 

DRAWING: C1

SHEET: 1 OF 31

AS-BUILT NO. 17-0300

# LEGAL DESCRIPTION:

### 102406,-9139,-9104 TINKER:

THE WEST 226 FEET OF THE SOUTH 185 FEET OF THE EAST HALF OF THE WEST HALF OF THE NORTHEAST COUNTY, WASHINGTON; EXCEPT THE SOUTH 30 FEET THEREOF.

THE EAST HALF OF THE SOUTHWEST QUARTER OF THE NORTHEAST QUARTER OF THE NORTHEAST QUARTER OF

### 102406-9180 ROCHFORD:

### 102406-9038 GORHAM:

PARCEL A: THE NORTH HALF OF THE NORTHWEST QUARTER OF THE NORTHWEST QUARTER OF THE FAST, W.M., IN KING COUNTY, WASHINGTON: EXCEPT THE NORTH 30 FEET; AND EXCEPT THE EAST 30 FEET THEREOF CONVEYED TO KING

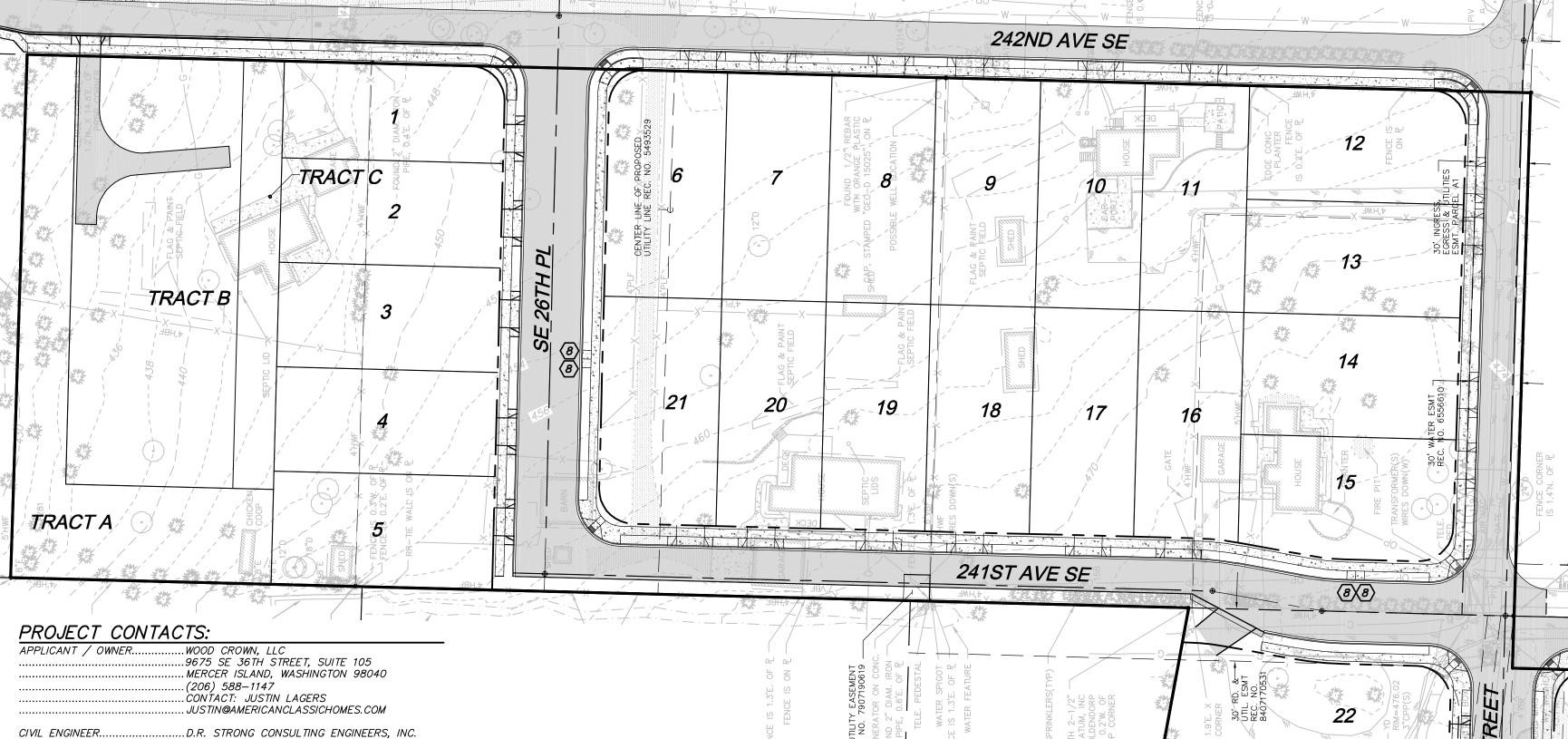
THE NORTH 30 FEET OF THE NORTH HALF OF THE NORTHWEST QUARTER OF THE NORTHWEST 24 NORTH, RANGE 6 EAST, W.M., IN KING COUNTY, WASHINGTON; EXCEPT THE EAST 30 FEET THEREOF CONVEYED TO KING COUNTY FOR ROADWAY PURPOSES UNDER RECORDING NUMBER

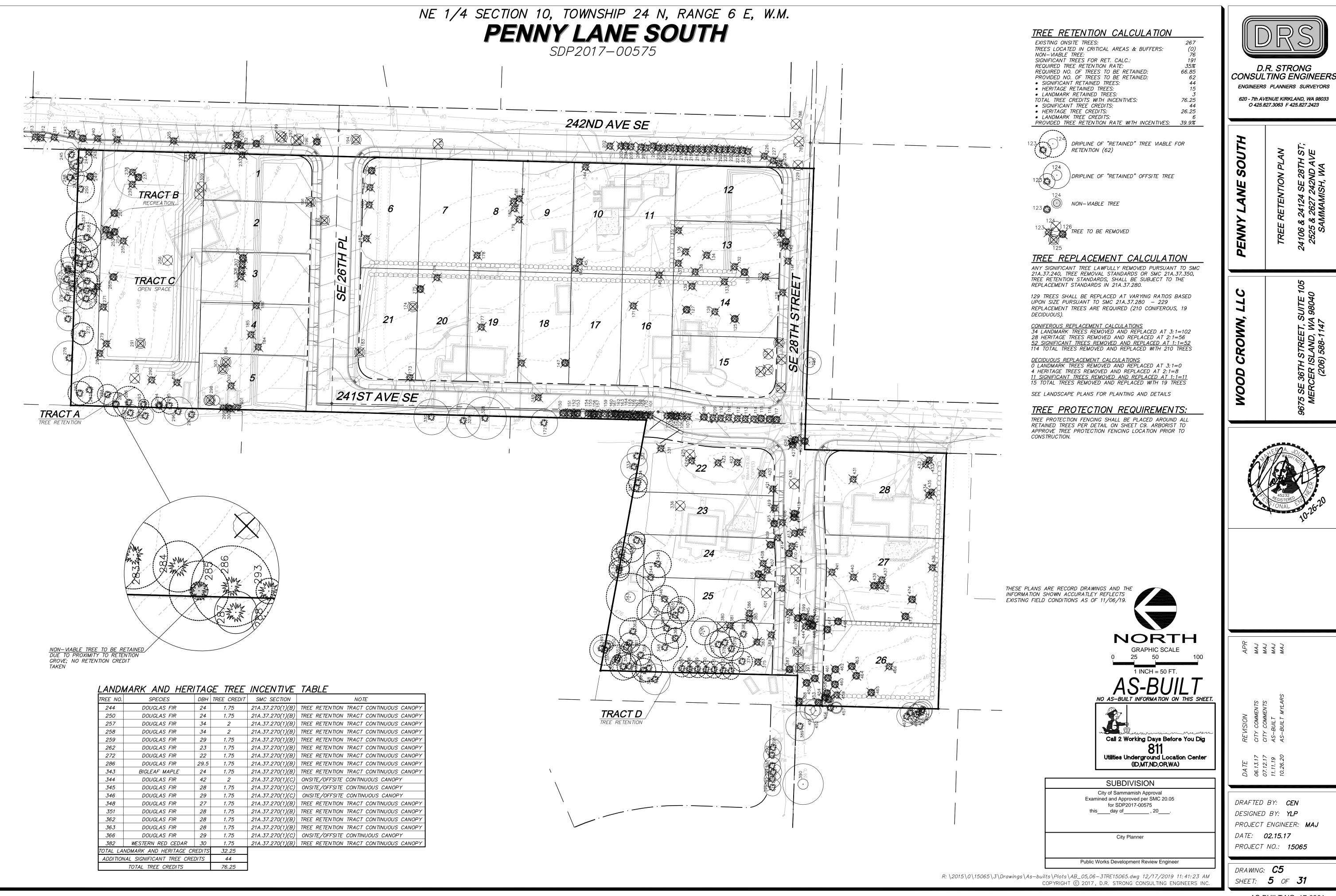
### 425-295-0625 TESC SUPERVISOR: (425) 449-4704

NAD 1983/91

### MAILBOX LOCATION:

4-8 UNIT (TYPE 1) N.D.C.B.U. LOCATION AS SHOWN IN PLAN. LOCATION APPROVED BY POSTMASTER. SEE DETAIL ON SHEET C17





# PENNY LANE SOUTH SDP2017-00575

TREE TABLE

TREE	TABLE						
TREE NO.	SPECIES	DBH	ADJ. DBH	DRIPLINE	HEALTH	REPLACEMENT TREES	REPLACEMENT SPECIES
101	DOUGLAS FIR	28.5	28.5	21	VIABLE	2	C
102	DOUGLAS FIR	27.5	27.5	21	VIABLE	2	C
103	DOUGLAS FIR	10	10	16	VIABLE	1	С
104	WESTERN RED CEDAR	20	20	21	VIABLE	1	С
105	DOUGLAS FIR	25	25	16	VIABLE	2	С
106	DOUGLAS FIR	18	18	12	VIABLE	1	С
107	DOUGLAS FIR	28	28	12	VIABLE	2	С
108	DOUGLAS FIR	24	24	12	NON-VIABLE		
109	DOUGLAS FIR	18	18	12	VIABLE	1	С
110	DOUGLAS FIR	16	16	12	VIABLE	1	С
111	DOUGLAS FIR	16	16	12	VIABLE	1	С
112	DOUGLAS FIR	15/15	21	12	VIABLE	1	С
113	DOUGLAS FIR	15	15	12	VIABLE	1	С
114	DOUGLAS FIR	15	15	12	VIABLE	1	С
115	DOUGLAS FIR	14	14	12	NON-VIABLE		
116	DOUGLAS FIR	16	16	12	NON-VIABLE		
117	DOUGLAS FIR	28	28	12	NON-VIABLE		
118	ASPEN	14	14	10	VIABLE	1	D
120	ASPEN	18	18	12	NON-VIABLE		
121	ASPEN	17	17	12	VIABLE	1	D
122	ASPEN	26	26	12	VIABLE	2	D
123	ASPEN	28	28	13	VIABLE	2	D
124	ASPEN	13	13	10	VIABLE	1	D
125	SPRUCE	18	18	15	NON-VIABLE		_
126	DOUGLAS FIR	34	34	21	VIABLE	3	С
127	DOUGLAS FIR	37	37	18	NON-VIABLE	-	_
128	DOUGLAS FIR	42 36	42 36	18	VIABLE	3	С
129	DOUGLAS FIR	36	36 36	16	VIABLE	3	С
130	DOUGLAS FIR	36 40	<i>36</i>	18	NON-VIABLE		
131 132	DOUGLAS FIR		40 30	21 16	NON-VIABLE	2	С
	DOUGLAS FIR	<i>30</i> <i>30</i>	<i>30</i> <i>30</i>	16 16	VIABLE VIABLE	2	C
133 134	DOUGLAS FIR  DOUGLAS FIR	<i>40</i>	40	16 21	VIABLE VIABLE	3	C
135	LODGEPOLE PINE	15	40 15	21 12	VIABLE VIABLE	1	C
135 136	DOUGLAS FIR	32	32	19	VIABLE VIABLE	3	C
137	DOUGLAS FIR	31	31	17	VIABLE	2	С
138	DOUGLAS FIR	30	30	15	VIABLE	2	C
139	DOUGLAS FIR	32.5	32.5	16	VIABLE	3	C
140	DOUGLAS FIR	35	35	18	VIABLE	3	С
141	DOUGLAS FIR	18	18	16	VIABLE	1	С
142	DOUGLAS FIR	23	23	17	VIABLE	2	С
143	DOUGLAS FIR	8/18	19.5	16	NON-VIABLE		
144	DOUGLAS FIR	26	26	20	VIABLE	2	С
145	DOUGLAS FIR	39	39	18	VIABLE	3	С
146	DOUGLAS FIR	35.5	<i>35.5</i>	20	VIABLE	3	С
147	DOUGLAS FIR	39	39	18	NON-VIABLE		
148	DOUGLAS FIR	32	32	18	VIABLE	3	С
149	DOUGLAS FIR	36	36	19	VIABLE	3	С
150	DOUGLAS FIR	12/12	17	15	NON-VIABLE		
151	DOUGLAS FIR	19	19	15	NON-VIABLE		
152	DOUGLAS FIR	24	24	15	VIABLE	2	С
153	DOUGLAS FIR	9	9	15	NON-VIABLE		
154	DOUGLAS FIR	15	15	15	VIABLE	1	С
155	DOUGLAS FIR	19	19	15	VIABLE	1	С
156	DOUGLAS FIR	31	31	15	NON-VIABLE		
157	DOUGLAS FIR	10	10	15	NON-VIABLE		
159	DOUGLAS FIR	24	24	15	VIABLE	2	С
160	DOUGLAS FIR	18	18	15	NON-VIABLE		
161	DOUGLAS FIR	12	12	15	NON-VIABLE		
162	DOUGLAS FIR	9	9	6	NON-VIABLE		
163	DOUGLAS FIR	8	8	6	NON-VIABLE		
164	DOUGLAS FIR	18	18	6	NON-VIABLE		
165	DOUGLAS FIR	27	27	6	NON-VIABLE	_	_
166	DOUGLAS FIR	8	8	6	VIABLE	1	С
167	DOUGLAS FIR	26	26	6	NON-VIABLE		_
168	DOUGLAS FIR	12	12	6	VIABLE	1	C
169	DOUGLAS FIR	16	16	15	VIABLE	1	С
170	DOUGLAS FIR	18	18 30	15	NON-VIABLE	7	
171	DOUGLAS FIR	<i>39</i>	39 40	18	VIABLE	3	C
173 174	DOUGLAS FIR	40 15	40 15	21	NON-VIARIE	3	С
174	RIVER BIRCH	15 20	15 20	14	NON-VIABLE		
175	DOUGLAS FIR	29	29	16	NON-VIABLE		<u> </u>

	TABLE						REPLACEMENT	REPLACEM
TREE NO.	SPECIES	DBH	ADJ. DBH	DRIPLINE	HEALTH	TREE CREDIT	TREES	SPECIES
176	DOUGLAS FIR	33	33	27	VIABLE		3	С
177	DOUGLAS FIR	48	48	30	VIABLE		3	С
178	DOUGLAS FIR	38.5	38.5	21	VIABLE		3	С
179	NOBLE FIR	11	11	5	VIABLE		1	С
180	NOBLE FIR	11	11	5	VIABLE		1	С
181	NOBLE FIR	8	8	8	VIABLE		1	С
182	NOBLE FIR	10	10	8	VIABLE		1	С
183	BIGLEAF MAPLE	20/22/27	40	30	NON-VIABLE			
184	WESTERN RED CEDAR	15	15	16	VIABLE		1	С
185	GRAND FIR	8	8	8	VIABLE		1	С
186	DOUGLAS FIR	16	16	12	VIABLE		1	С
187	SEQUOIA	54	54	15	VIABLE		3	С
188	SEQUOIA	47	47	15	VIABLE		3	С
189	SEQUOIA	41.5	41.5	15	VIABLE		3	С
190	SEQUOIA	43.5	43.5	15	NON-VIABLE			
191	BEECH	9/13	16	15	NON-VIABLE			
192	BEECH	15	15	15	NON-VIABLE			_
233	LODGEPOLE PINE	13	13	9	VIABLE		1	C
234	DOUGLAS FIR	29	29	20	VIABLE		2	С
238	DOUGLAS FIR	19	19	14	VIABLE		1	С
239	DOUGLAS FIR	30	30	18	VIABLE		2	С
243	DOUGLAS FIR	24	24	15	VIABLE		2	С
244	DOUGLAS FIR	24	24	14	VIABLE	1.75		
249	DOUGLAS FIR	24	24	10	NON-VIABLE			
250	DOUGLAS FIR	24	24	16	VIABLE	1.75		
251	FRAZIER	10	10	11	NON-VIABLE			
252	GRAND FIR	10.5	10.5	8	NON-VIABLE			
253	GRAND FIR	10	10	10	NON-VIABLE			
254	GRAND FIR	10	10	11	NON-VIABLE			
255	DOUGLAS FIR	32	32	15	VIABLE		3	С
256	OAK	23	23	29	VIABLE		2	D
<i>257</i>	DOUGLAS FIR	34	34	18	VIABLE	2		
258	DOUGLAS FIR	34	34	17	VIABLE	2		
259	DOUGLAS FIR	29	29	17	VIABLE	1.75		
260	GRAND FIR	9	9	10	VIABLE	1		
261	DOUGLAS FIR	21	21	18	VIABLE	1		
262	DOUGLAS FIR	23	23	9	VIABLE	1.75		
265	DOUGLAS FIR	27	27	18	NON-VIABLE			
266	DOUGLAS FIR	25	25	16	VIABLE		2	С
267	DOUGLAS FIR	16	16	16	VIABLE		1	С
268	GRAND FIR	12	12	12	VIABLE	1		
269	GRAND FIR	9	9	12	VIABLE	1		
270	GRAND FIR	10	10	12	VIABLE	1		
271	DOUGLAS FIR	30.5	30.5	19	VIABLE		2	С
272	DOUGLAS FIR	22	22	13	VIABLE	1.75		
279	DOUGLAS FIR	30	30	20	VIABLE		2	С
280	DOUGLAS FIR	31.5	31.5	23	VIABLE		2	С
282	DOUGLAS FIR	21	21	14	VIABLE	1		
283	DOUGLAS FIR	13	13	16	VIABLE	1		
284	DOUGLAS FIR	12	12	12	VIABLE	1		
285	DOUGLAS FIR	29.5	29.5	19	NON-VIABLE			
286	DOUGLAS FIR	29.5	29.5	19	VIABLE	1.75		
289	BIGLEAF MAPLE	20	20	28	VIABLE		1	D
290	DOUGLAS FIR	40	40	20	VIABLE		3	С
1		12	12	14	NON-VIABLE			
291	ALDER		1	16	VIABLE		3	С
291 292	ALDER DOUGLAS FIR	33	33	10				
			33 8	6	VIABLE	1		
292	DOUGLAS FIR	33			VIABLE VIABLE	1		
292 293	DOUGLAS FIR  DOUGLAS FIR	33 8	8	6				
292 293 294 295	DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR	33 8 8 11	<i>8</i>	6 9	VIABLE	1		
292 293 294 295	DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR	33 8 8 11	8 8 11	6 9 10	VIABLE VIABLE	1	1	С
292 293 294 295 298	DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  BIGLEAF MAPLE CLUSTER	33 8 8 11 ? 18/9/7	8 8 11 21.5	6 9 10 24	VIABLE VIABLE NON-VIABLE	1	1	<i>C C</i>
292 293 294 295 298 299	DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  BIGLEAF MAPLE CLUSTER  DOUGLAS FIR	33 8 8 11 11 18/9/7	8 8 11 21.5 14	6 9 10 24 10	VIABLE  VIABLE  NON-VIABLE  VIABLE	1		
292 293 294 295 298 299 300	DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  BIGLEAF MAPLE CLUSTER  DOUGLAS FIR  DOUGLAS FIR	33 8 8 11 11 18/9/7 14 14	8 8 11 21.5 14 14	6 9 10 24 10 10	VIABLE  VIABLE  NON-VIABLE  VIABLE  VIABLE	1	1	С
292 293 294 295 298 299 300 301	DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  BIGLEAF MAPLE CLUSTER  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR	33 8 8 11 18/9/7 14 14 17	8 8 11 21.5 14 14 17	6 9 10 24 10 10 16	VIABLE VIABLE  NON-VIABLE VIABLE VIABLE VIABLE	1	1	<i>C C</i>
292 293 294 295 298 299 300 301 302	DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  BIGLEAF MAPLE CLUSTER  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR	33 8 8 11 18/9/7 14 14 17 33	8 8 11 21.5 14 14 17 33	6 9 10 24 10 10 16 15	VIABLE  VIABLE  NON – VIABLE  VIABLE  VIABLE  VIABLE  VIABLE	1	1	<i>C C</i>
292 293 294 295 298 299 300 301 302 303	DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  BIGLEAF MAPLE CLUSTER  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  ALDER	33 8 8 11 18/9/7 14 14 17 33 12	8 8 11 21.5 14 14 17 33 12	6 9 10 24 10 10 16 15 14	VIABLE VIABLE  VIABLE VIABLE VIABLE VIABLE VIABLE VIABLE VIABLE	1	1	<i>C C</i>
292 293 294 295 298 299 300 301 302 303 304	DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  BIGLEAF MAPLE CLUSTER  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  ALDER  ALDER	33 8 8 11 18/9/7 14 14 17 33 12 16	8 8 11 21.5 14 14 17 33 12 16	6 9 10 24 10 10 16 15 14	VIABLE VIABLE  VIABLE VIABLE VIABLE VIABLE VIABLE VIABLE NON-VIABLE	1	1 1 3	C C C
292 293 294 295 298 299 300 301 302 303 304 305	DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  BIGLEAF MAPLE CLUSTER  DOUGLAS FIR  DOUGLAS FIR  DOUGLAS FIR  ALDER  ALDER  DOUGLAS FIR	33 8 8 11 18/9/7 14 17 33 12 16 20	8 8 11 21.5 14 14 17 33 12 16 20	6 9 10 24 10 10 16 15 14 14	VIABLE  VIABLE  VIABLE  VIABLE  VIABLE  VIABLE  VIABLE  VIABLE  NON-VIABLE  VIABLE  VIABLE	1	1 3 3	C C C

JAPANESE MAPLE 7/7/9 13.5 16

JAPANESE MAPLE | 10/10/12 | 18.5 | 16

TRF	F	TAF	RIF	-

REE NO.	SPECIES	DBH	ADJ. DBH	DRIPLINE	HEALTH	TREE CREDIT	REPLACEMENT TREES	SPECIES
331	DOUGLAS FIR	17	17	15	VIABLE		1	С
334	WESTERN RED CEDAR	8/12	14.5	11	VIABLE	1		
335	WESTERN RED CEDAR	12	12	12	VIABLE	1		
336	BIRCH	14/15	20.5	18	VIABLE		1	D
<i>337</i>	DOUGLAS FIR	9	9	10	VIABLE	1		
338	CHERRY	14	14	16	VIABLE	1		
339	WESTERN RED CEDAR	13	13	12	VIABLE	1		
	WESTERN RED CEDAR	11	11	12	VIABLE	1		
	WESTERN RED CEDAR	10	10	12	<u>VIABLE</u>	1		
	WESTERN RED CEDAR	10	10	12	VIABLE	1 75		
343	BIGLEAF MAPLE	24	24	28	VIABLE	1.75		
<i>344 345</i>	DOUGLAS FIR  DOUGLAS FIR	42 28	42 28	17 16	VIABLE VIABLE	2 1.75		
346	DOUGLAS FIR	29	29	16	VIABLE	1.75		
347	BIGLEAF MAPLE	20	20	20	VIABLE	1		
348	DOUGLAS FIR	27	27	16	VIABLE	1.75		
350	DOUGLAS FIR	8	8	6	VIABLE	1		
351	DOUGLAS FIR	28	28	18	VIABLE	1.75		
352	DOUGLAS FIR	12	12	15	VIABLE	1		
353	DOUGLAS FIR	8	8	6	VIABLE	1		
354	DOUGLAS FIR	15	15	16	VIABLE	1		
<i>355</i>	DOUGLAS FIR	8	8	14	VIABLE	1		
	WESTERN RED CEDAR		20	14	VIABLE	1		
<i>357</i>	WESTERN RED CEDAR	14	14	16	VIABLE	1		
359	DOUGLAS FIR	21	21	12	<u>VIABLE</u>	1		
360	DOUGLAS FIR	8	8	6	<u>VIABLE</u>	1		
361	DOUGLAS FIR	11	11	12	VIABLE	1 75		
362	DOUGLAS FIR	28	28	16	VIABLE	1.75		
<i>363 364</i>	DOUGLAS FIR	28 12	28 12	18 12	VIABLE VIABLE	1.75 1		
365	WESTERN RED CEDAR  DOUGLAS FIR	21	21	16	VIABLE	1		
366	DOUGLAS FIR	29	29	16	VIABLE	1. 75		
	LEYLANDII CYPRESS	4/8/5	10	10	VIABLE	1		
		6/5/3/4		10	VIABLE	1		
	LEYLANDII CYPRESS	9	9	10	VIABLE	1		
370	LEYLANDII CYPRESS	7/5/4/3	10	10	VIABLE	1		
371	LEYLANDII CYPRESS	9	9	10	VIABLE	1		
372	LEYLANDII CYPRESS	9	9	10	VIABLE	1		
373	LEYLANDII CYPRESS	7/7	10	10	VIABLE	1		
374	WESTERN RED CEDAR	14	14	12	VIABLE	1		
<i>375</i>	WESTERN RED CEDAR	22	22	15	VIABLE		2	С
376	DOUGLAS FIR	18	18	16	VIABLE	1		
	WESTERN RED CEDAR	16	16	16	<u>VIABLE</u>	1		
	WESTERN RED CEDAR	20	20	18	VIABLE	1		
	WESTERN RED CEDAR	20	20	14	VIABLE	1	4	
380	GOLDEN LOCUST WESTERN RED CEDAR	11 10	11	18 10	VIABLE		1	D C
	WESTERN RED CEDAR				VIABLE VIABLE	1 75	, , , , , , , , , , , , , , , , , , ,	C
	WESTERN RED CEDAR		30 29	17	VIABLE	1.75	2	С
	WESTERN RED CEDAR	<u> </u>	35	14	VIABLE		3	С
	WESTERN RED CEDAR	18	18	14	VIABLE		1	С
	WESTERN RED CEDAR	33	33	14	VIABLE		3	С
<i>387</i>	DOUGLAS FIR	19	19	14	NON-VIABLE			
388	DOUGLAS FIR	22	22	16	NON-VIABLE			
<i>397</i>	BIGLEAF MAPLE	12/11/9	18.5	16	NON-VIABLE			
398	BIGLEAF MAPLE	13	13	16	NON-VIABLE			
399	WESTERN RED CEDAR	13	13	14	NON-VIABLE			
400	HEMLOCK	13	13	14	NON-VIABLE			
401		8/9/9/13		18	VIABLE		1	D
	WESTERN RED CEDAR		19	14	VIABLE		1	С
	WESTERN RED CEDAR		21	14	VIABLE		1	<i>C</i>
404	CHERRY	12	12	16	VIABLE		1	D
405	DOUGLAS FIR	15 20	15	12	VIABLE		1	С
406 407	DOUGLAS FIR	20	20	16	NON-VIABLE		1	
407 408	DOUGLAS FIR	14 26	14	15 15	VIABLE		2	<u>с</u>
408	DOUGLAS FIR  DOUGLAS FIR	26 12	26 12	15 12	VIABLE NON-VIABLE			L C
410	DOUGLAS FIR	10	10	8	NON-VIABLE			
	WESTERN RED CEDAR	20	20	14	VIABLE		1	С
•		-		· ·	<del>_</del>			
412	DOUGLAS FIR	16	16	12	VIABLE		1	C
	DOUGLAS FIR WESTERN RED CEDAR		16 15	12 12	VIABLE VIABLE		1	<i>C</i>

414 WESTERN RED CEDAR 15 15 14 VIABLE

### TREE TABLE

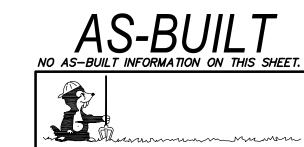
TREE NO.	SPECIES	DBH	ADJ. DBH	DRIPLINE	HEALTH	REPLACEMENT TREES	REPLACEMENT SPECIES
415	WESTERN RED CEDAR	32	32	17	NON-VIABLE		
416	WESTERN RED CEDAR	12	12	13	VIABLE	1	С
417	DOUGLAS FIR	16	16	12	NON-VIABLE		
418	DOUGLAS FIR	13	13	12	NON-VIABLE		
419	DOUGLAS FIR	15	15	14	VIABLE	1	С
420	DOUGLAS FIR	18	18	22	VIABLE	1	С
421	DOUGLAS FIR	17	17	16	VIABLE	1	С
422	WESTERN RED CEDAR	10/9/9	16	9	VIABLE	1	С
423	WESTERN RED CEDAR	16/14/12	24.5	9	VIABLE	2	С
424	WESTERN RED CEDAR	13/12	17.5	9	VIABLE	1	С
425	CRAB APPLE	12	12	18	VIABLE	1	D
426	DOUGLAS FIR	19	19	20	VIABLE	1	С
427	DOUGLAS FIR	11	11	13	NON-VIABLE		
428	DOUGLAS FIR	15	15	20	NON-VIABLE		
429	DOUGLAS FIR	24	24	20	NON-VIABLE		
430	BIGLEAF MAPLE	24	24	22	VIABLE	2	D
431	WESTERN RED CEDAR	39	39	22	VIABLE	3	С
432	WESTERN RED CEDAR	32	32	16	VIABLE	3	С
433	WESTERN RED CEDAR	33	33	18	VIABLE	3	С
434	WESTERN RED CEDAR	34	34	17	VIABLE	3	С
435	WESTERN RED CEDAR	37	37	16	VIABLE	3	С
436	DOUGLAS FIR	30	30	14	VIABLE	2	С
437	DOUGLAS FIR	35	35	14	VIABLE	3	С
438	DOUGLAS FIR	29	29	16	VIABLE	2	С
439	DOUGLAS FIR	35	35	16	NON-VIABLE		
440	DOUGLAS FIR	39	39	16	VIABLE	3	С
441	DOUGLAS FIR	36	36	19	VIABLE	3	С
442	WESTERN RED CEDAR	10	10	12	VIABLE	1	С
443	WESTERN RED CEDAR	12	12	16	VIABLE	1	С
444	WESTERN RED CEDAR	10	10	12	VIABLE	1	С
445	WESTERN RED CEDAR	16	16	14	VIABLE	1	С
446	WESTERN RED CEDAR	13/11	17	13	NON-VIABLE		
447	DOUGLAS FIR	29	29	16	NON-VIABLE		
448	DOUGLAS FIR	29	29	20	NON-VIABLE		
449	DOUGLAS FIR	14	14	10	NON-VIABLE		
450	DOUGLAS FIR	8	8	10	NON-VIABLE		
453	DOUGLAS FIR	14	14	12	NON-VIABLE		
454	DOUGLAS FIR	10	10	8	NON-VIABLE		
455	DOUGLAS FIR	10	10	6	NON-VIABLE		
458	DOUGLAS FIR	20	20	10	NON-VIABLE		
459	DOUGLAS FIR	24	24	12	VIABLE	2	С
460	DOUGLAS FIR	20	20	6	NON-VIABLE		
461	DOUGLAS FIR	21	21	10	NON-VIABLE		
462	DOUGLAS FIR	26	26	12	NON-VIABLE		
463	DOUGLAS FIR	24	24	14	NON-VIABLE		
464	DOUGLAS FIR	22	22	14	NON-VIABLE		
465	DOUGLAS FIR	39	39	14	NON-VIABLE		
466	WESTERN RED CEDAR	36	36	14	VIABLE	3	С
473	DOUGLAS FIR	24	24	15	NON-VIABLE		
474	HEMLOCK	28	28	18	NON-VIABLE		

TREE TABLE NOTE

1. DBH = DIAMETER BREAST HEIGHT IN INCHES
2. DRIPLINE = RADIUS IN FEET

RETAINED TREE

THESE PLANS ARE RECORD DRAWINGS AND THE INFORMATION SHOWN ACCURATLEY REFLECTS EXISTING FIELD CONDITIONS AS OF 11/06/19.



Working Days 2811
Utilities Underground Location Center (ID,MT,ND,OR,WA)

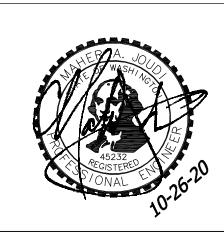
SUBDIVISION City of Sammamish Approval Examined and Approved per SMC 20.05 for SDP2017-00575 this\_\_\_\_day of\_\_\_\_

City Planner

Public Works Development Review Engineer R:  $\2015\0\15065\3\Drawings\As-builts\Plots\AB\_05,06-3TRE15065.dwg$  12/17/2019 11:41:23 AM COPYRIGHT © 2017, D.R. STRONG CONSULTING ENGINEERS INC.

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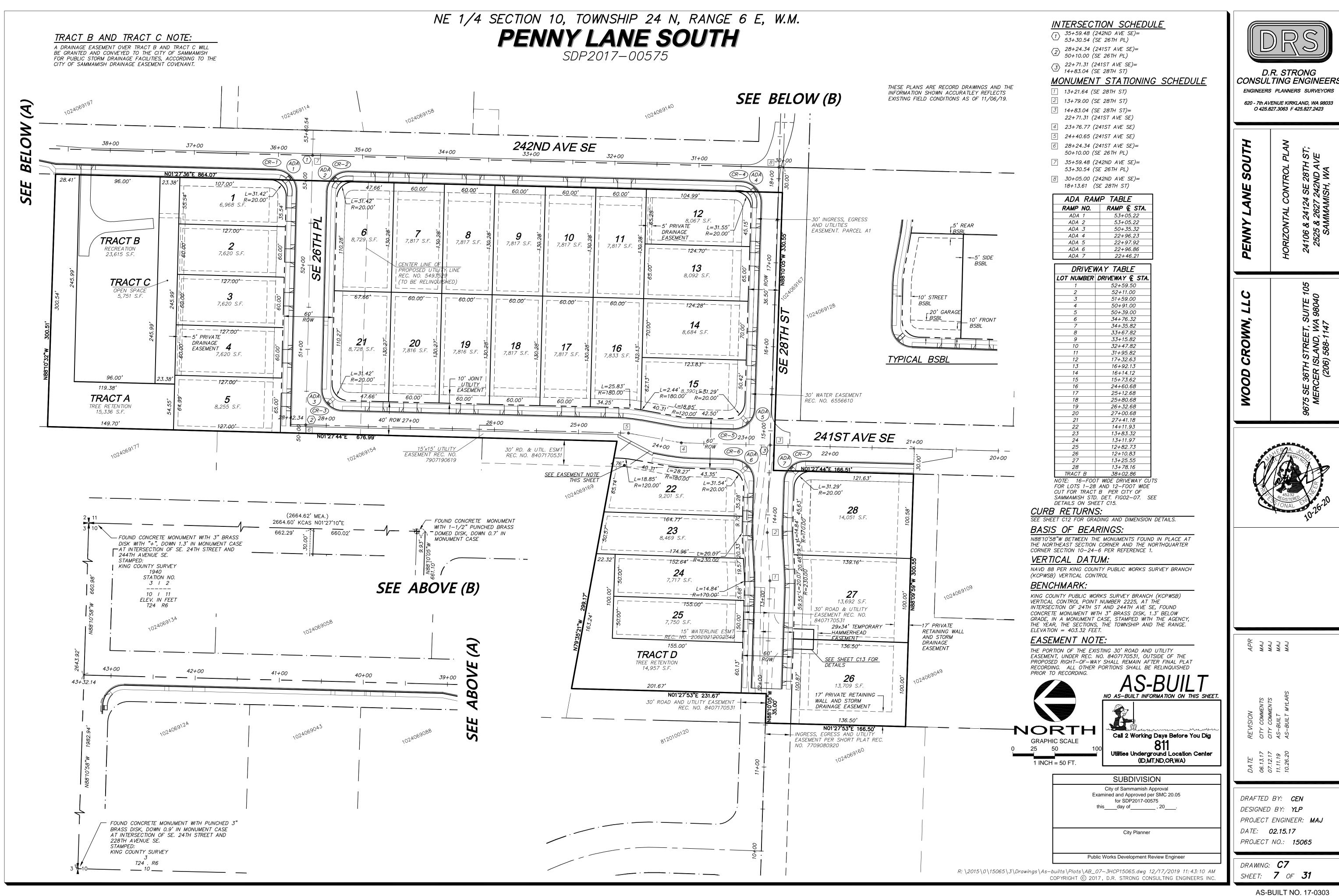


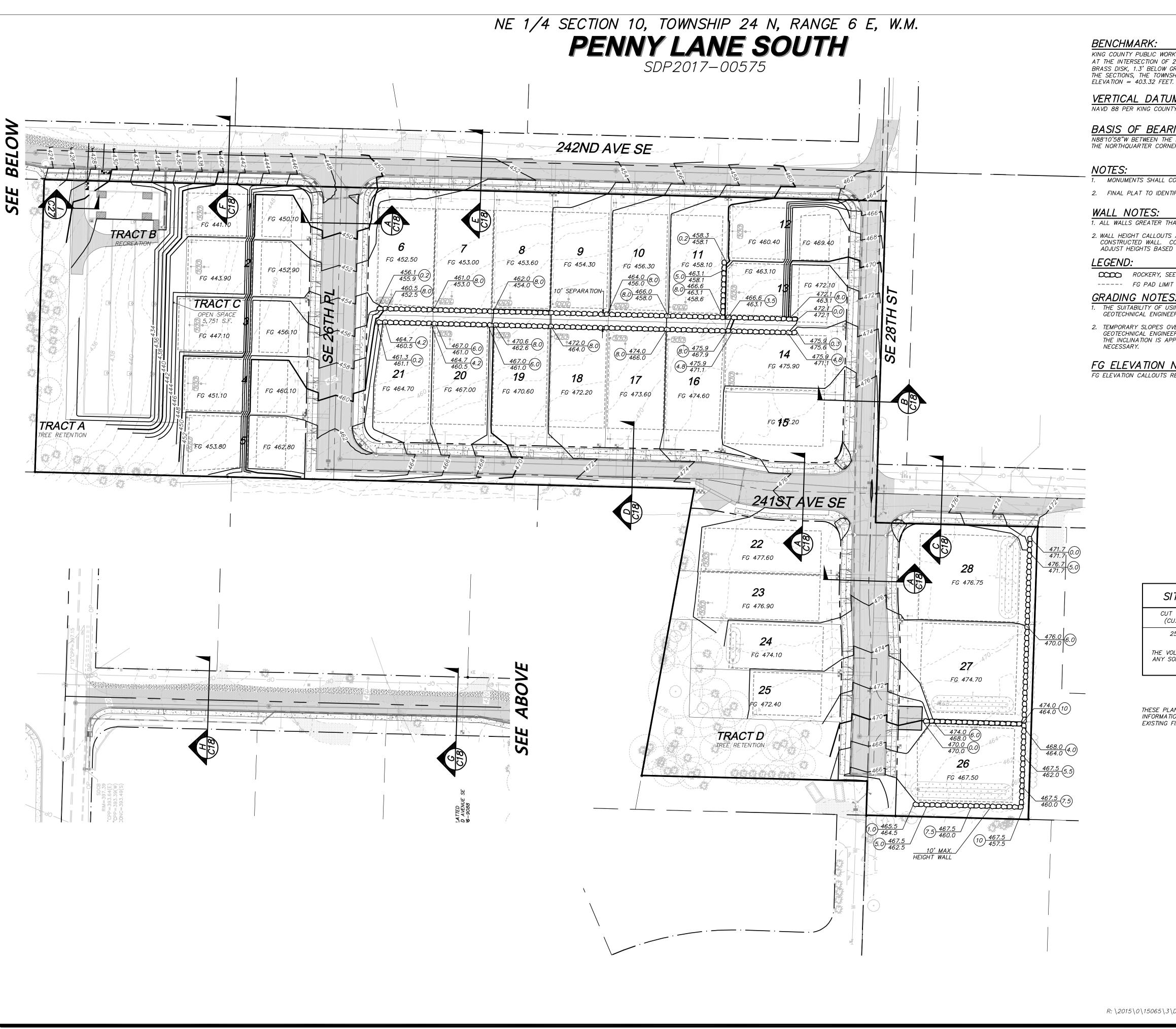
APR MAS MAS MAS

DATE 06.13.17 07.12.17 11.11.19 10.26.20

DRAFTED BY: **CEN** DESIGNED BY: YLP PROJECT ENGINEER: MAJ DATE: **02.15.17** PROJECT NO.: **15065** 

DRAWING: **C6** SHEET: **6** OF **31** 





KING COUNTY PUBLIC WORKS SURVEY BRANCH (KCPWSB) VERTICAL CONTROL POINT NUMBER 2225, AT THE INTERSECTION OF 24TH ST AND 244TH AVE SE, FOUND CONCRETE MONUMENT WITH 3" BRASS DISK, 1.3' BELOW GRADE, IN A MONUMENT CASE, STAMPED WITH THE AGENCY, THE YEAR, THE SECTIONS, THE TOWNSHIP AND THE RANGE. ELEVATION = 403.32 FEET.

### VERTICAL DATUM:

NAVD 88 PER KING COUNTY PUBLIC WORKS SURVEY BRANCH (KCPWSB) VERTICAL CONTROL

### BASIS OF BEARINGS:

N88'10'58"W BETWEEN THE MONUMENTS FOUND IN PLACE AT THE NORTHEAST SECTION CORNER AND THE NORTHQUARTER CORNER SECTION 10-24-6 PER REFERENCE 1.

MONUMENTS SHALL CONFORM TO DETAIL PER SHEET C17.

2. FINAL PLAT TO IDENTIFY LOT CORNERS AND OTHER MONUMENTATION.

2. WALL HEIGHT CALLOUTS ARE FINISHED GRADE TO FINISHED GRADE, NOT TO BOTTOM OF CONSTRUCTED WALL. CONTRACTOR MUST ACCOUNT FOR MINIMUM BURY AND MAY NEED TO ADJUST HEIGHTS BASED UPON WALL UNIT DIMENSIONS.

ROCKERY, SEE DETAIL ON SHEET R1

### **GRADING NOTES:**

1. THE SUITABILITY OF USING THE ON-SITE SOILS AS STRUCTURAL FILL SHALL BE EVALUATED BY A

2. TEMPORARY SLOPES OVER FOUR FEET IN HEIGHT SHALL BE NO STEEPER THAN 1.5H:1V. A GEOTECHNICAL ENGINEER SHALL OBSERVE TEMPORARY AND PERMANENT SLOPES TO VERIFY THAT THE TRANSPORTED THE CONDITIONS EXPOSED. TEMPORARY SHORING MAY BE

### FG ELEVATION NOTE:

FG ELEVATION CALLOUTS REFER TO FINISHED GRADE ELEVATION, NOT STRUCTURAL PAD ELEVATION.

### SITE VOLUME CALCULATIONS

CUT VOLUME FILL VOLUME NET VOLUME (CU. YDS.) (CU. YDS.) (CU. YDS.) 22,937 2,475 EXPORT

ALL VOLUMES ARE APPROXIMATE. THE VOLUMES DO NOT INCLUDE EXPANSION FACTOR OR ANY SOIL TYPE RESTRICTIONS. THE VOLUMES INCLUDE STRUCTURAL EXCAVATION FOR VAULT.

THESE PLANS ARE RECORD DRAWINGS AND THE INFORMATION SHOWN ACCURATLEY REFLECTS EXISTING FIELD CONDITIONS AS OF 11/06/19.

NORTH GRAPHIC SCALE 25 50

1 INCH = 50 FT. NO AS-BUILT INFORMATION ON THIS SHEET.

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Utilities Underground Location Center (ID,MT,ND,OR,WA)

### SUBDIVISION

City of Sammamish Approval Examined and Approved per SMC 20.05 for SDP2017-00575 this\_\_\_\_day of\_\_\_

City Planner

Public Works Development Review Engineer

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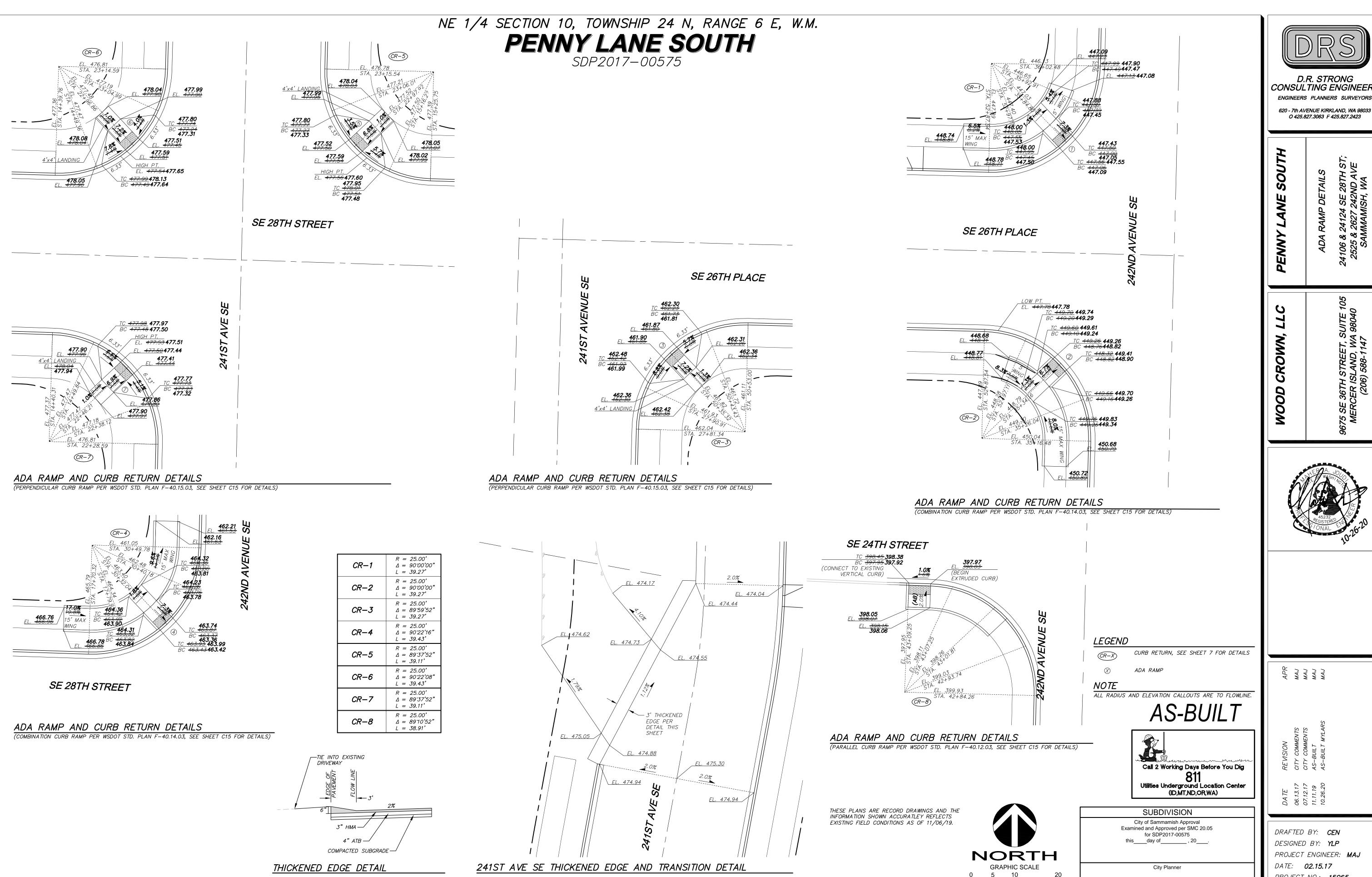


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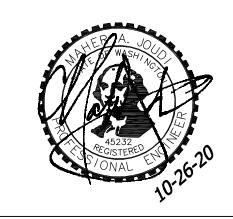
7E 13. 12. 11.1 DA 06. 07. 11.11

DRAFTED BY: CEN DESIGNED BY: YLP PROJECT ENGINEER: MAJ DATE: **02.15.17** PROJECT NO.: **15065** 

DRAWING: C11 SHEET: 11 OF 31



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47E .13.17 .12.17 11.19 26.20 DA 06.1 07.1 11.1

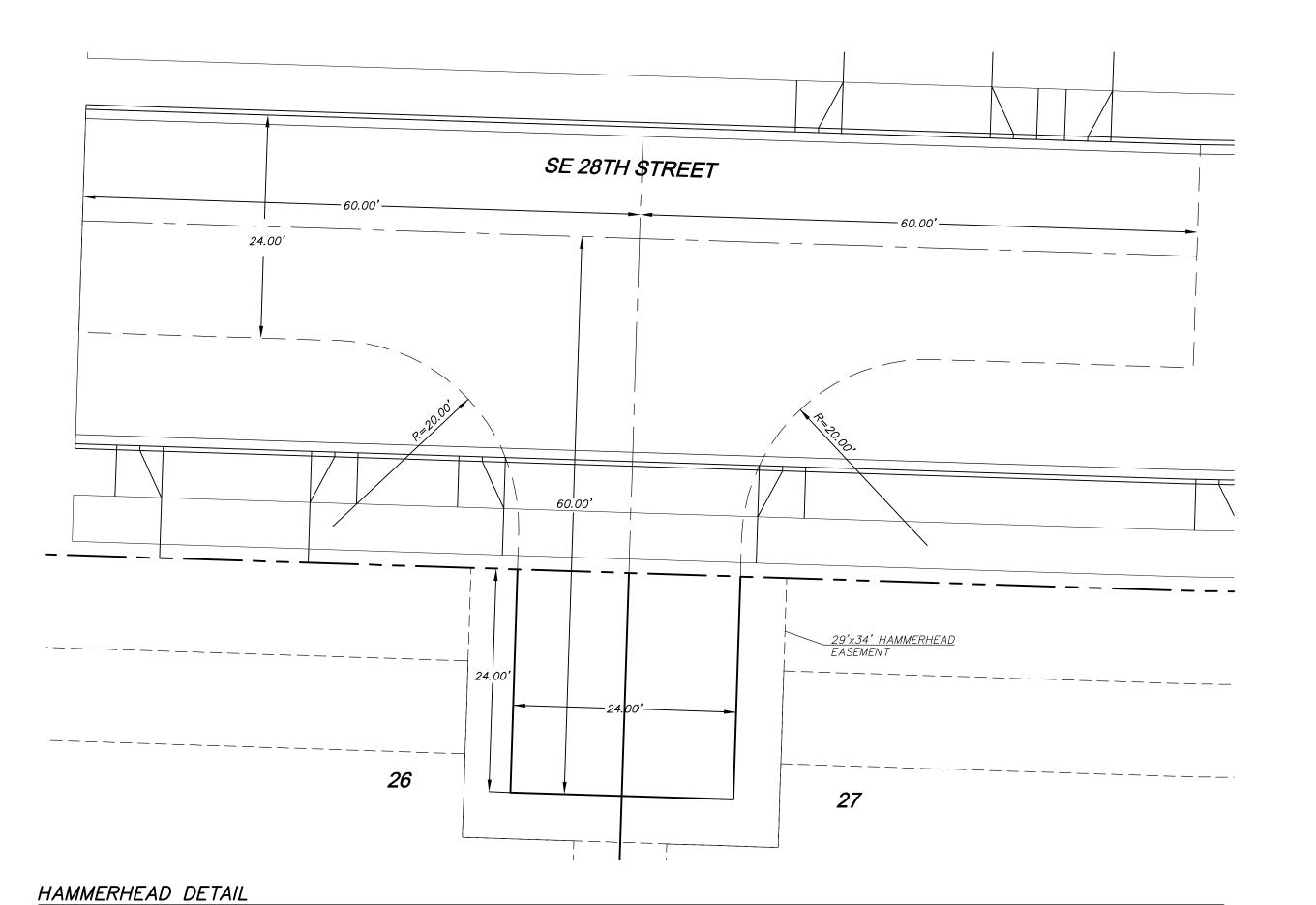
DRAFTED BY: CEN DESIGNED BY: YLP PROJECT ENGINEER: MAJ DATE: **02.15.17** PROJECT NO.: **15065** 

DRAWING: C12 SHEET: 12 OF 31

Public Works Development Review Engineer

1 INCH = 10 FT.

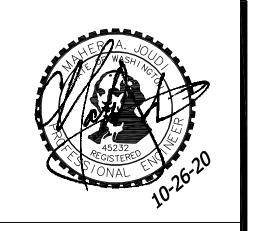
# NE 1/4 SECTION 10, TOWNSHIP 24 N, RANGE 6 E, W.M. PENNY LANE SOUTH SDP2017-00575





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AS-BUILT NO AS-BUILT INFORMATION ON THIS SHEET.

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Utilities Underground Location Center
(ID,MT,ND,OR,WA)

SUBDIVISION City of Sammamish Approval Examined and Approved per SMC 20.05

for SDP2017-00575

City Planner

Public Works Development Review Engineer

this\_\_\_\_day of\_\_\_\_

DATE 06.13.17 07.12.17 11.11.19 10.26.20

DRAFTED BY: **CEN** DESIGNED BY: YLP PROJECT ENGINEER: MAJ DATE: **02.15.17** 

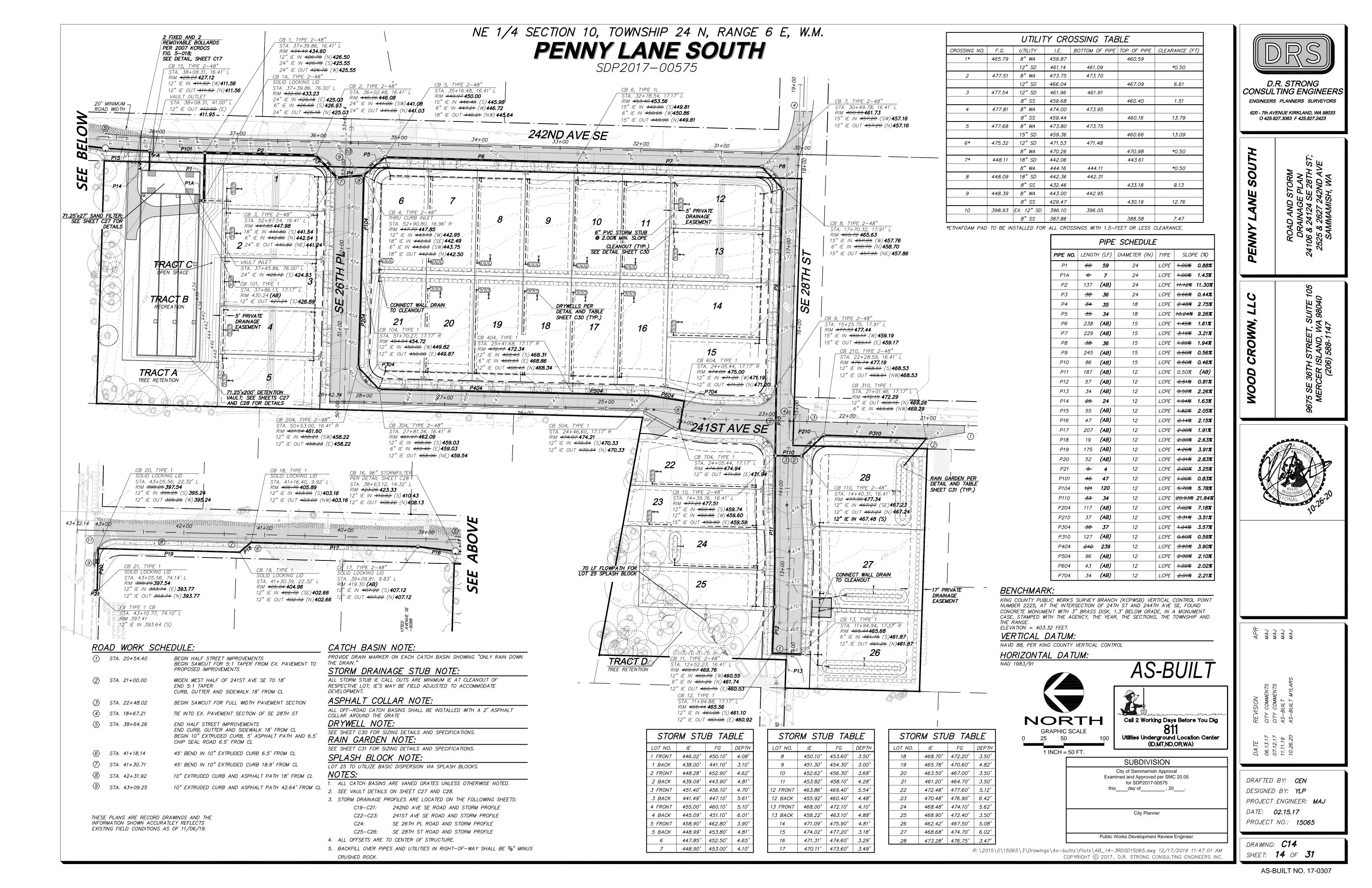
PROJECT NO.: **15065** DRAWING: C13 SHEET: 13 OF 31

NORTH GRAPHIC SCALE

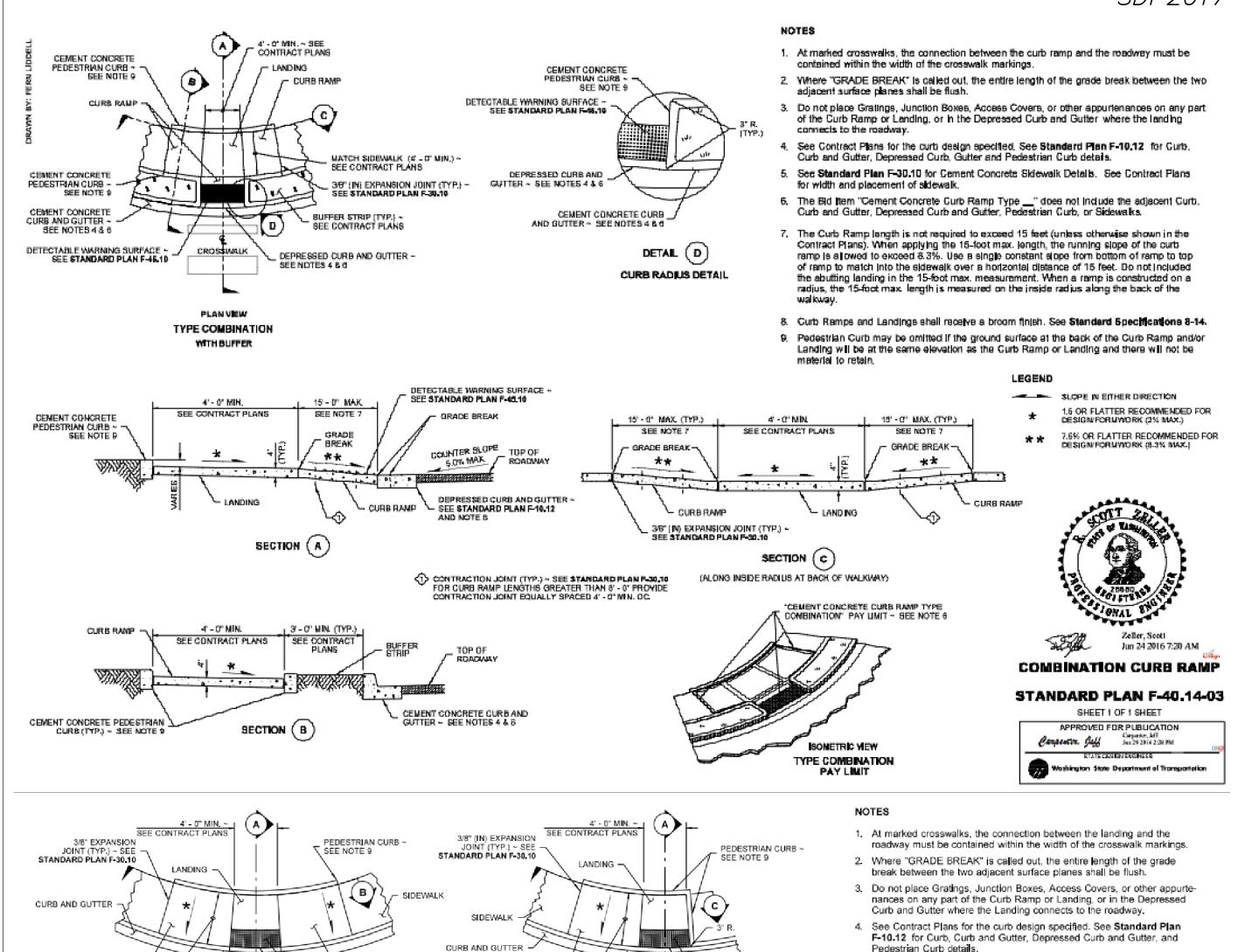
1 INCH = 10 FT.

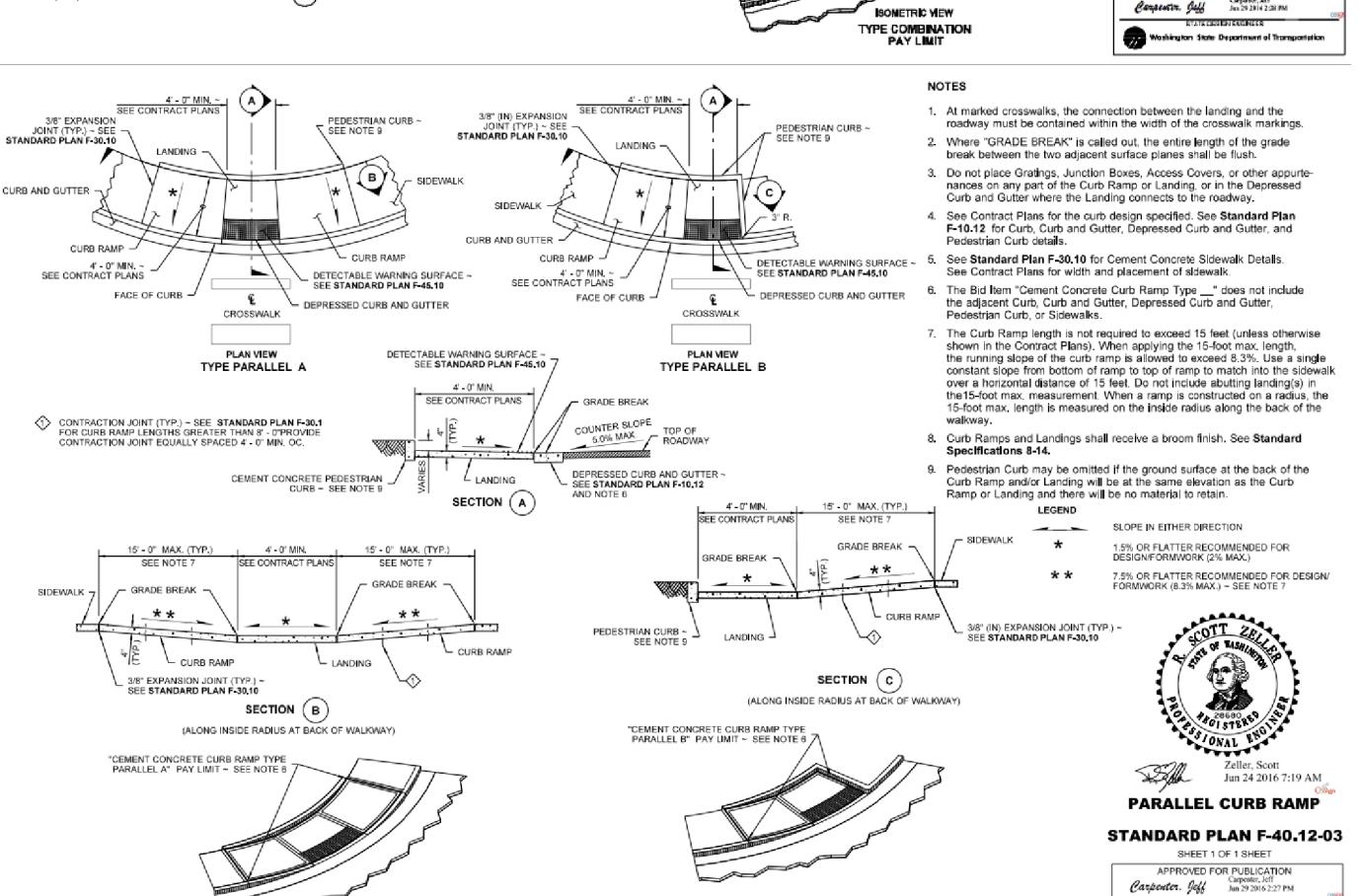
THESE PLANS ARE RECORD DRAWINGS AND THE INFORMATION SHOWN ACCURATLEY REFLECTS EXISTING FIELD CONDITIONS AS OF 11/06/19.

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## PENNY LANE SOUTH



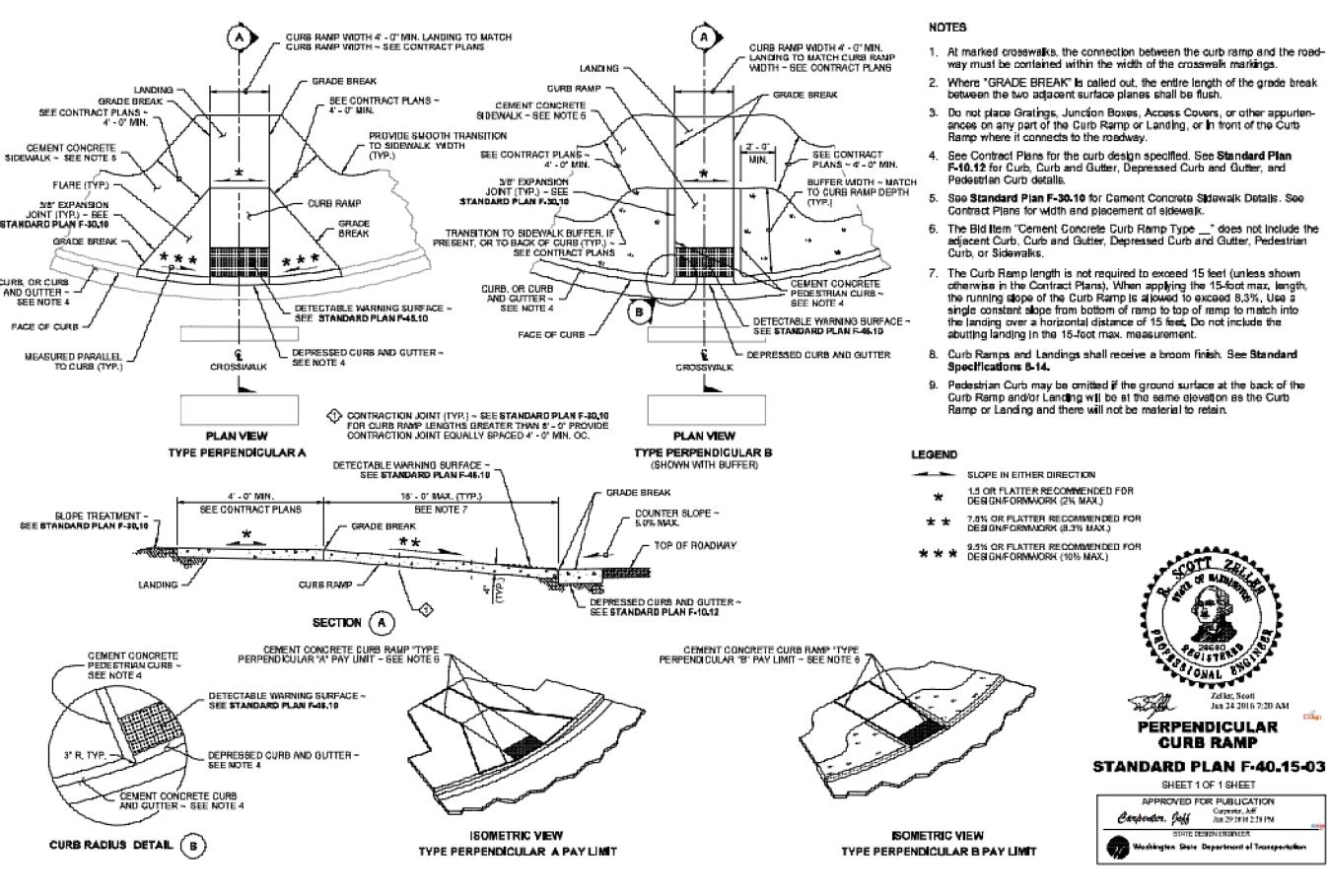


TYPE PARALLEL A PAY LIMIT

ISOMETRIC VIEW

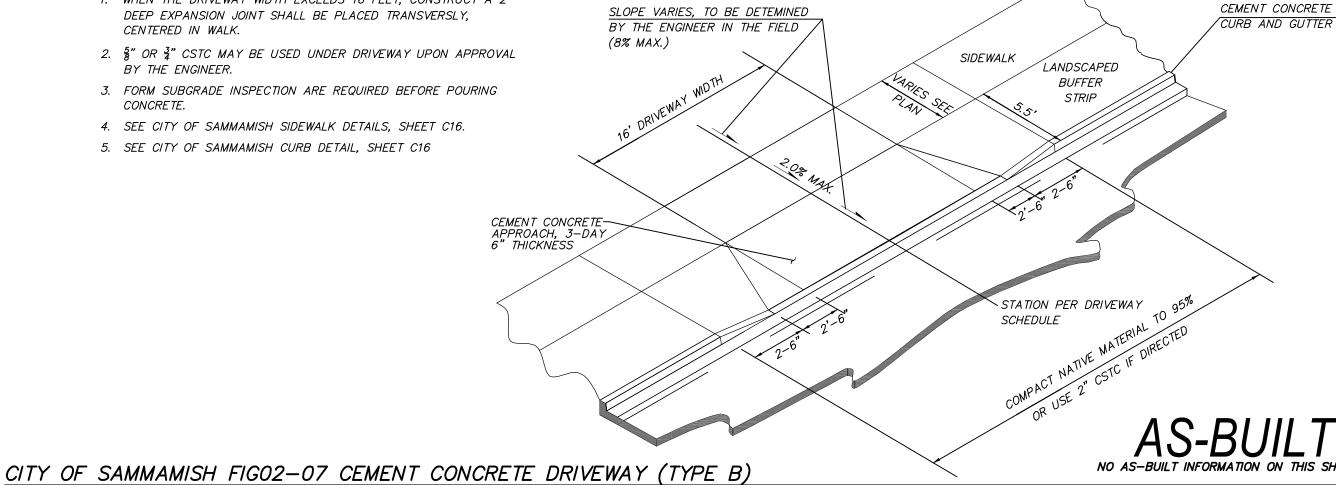
TYPE PARALLEL B PAY LIMIT

Washington State Department of Transportation





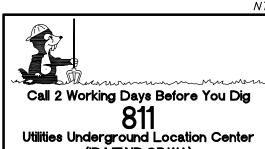
- 1. WHEN THE DRIVEWAY WIDTH EXCEEDS 16 FEET, CONSTRUCT A 2" DEEP EXPANSION JOINT SHALL BE PLACED TRANSVERSLY, CENTERED IN WALK.
- 2. 5" OR 3" CSTC MAY BE USED UNDER DRIVEWAY UPON APPROVAL
- 3. FORM SUBGRADE INSPECTION ARE REQUIRED BEFORE POURING CONCRETE.
- 4. SEE CITY OF SAMMAMISH SIDEWALK DETAILS, SHEET C16.
- 5. SEE CITY OF SAMMAMISH CURB DETAIL, SHEET C16



INFORMATION SHOWN ACCURATLEY REFLECTS

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(ID.MT.ND.OR.WA)

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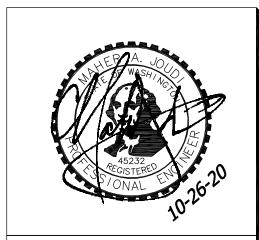
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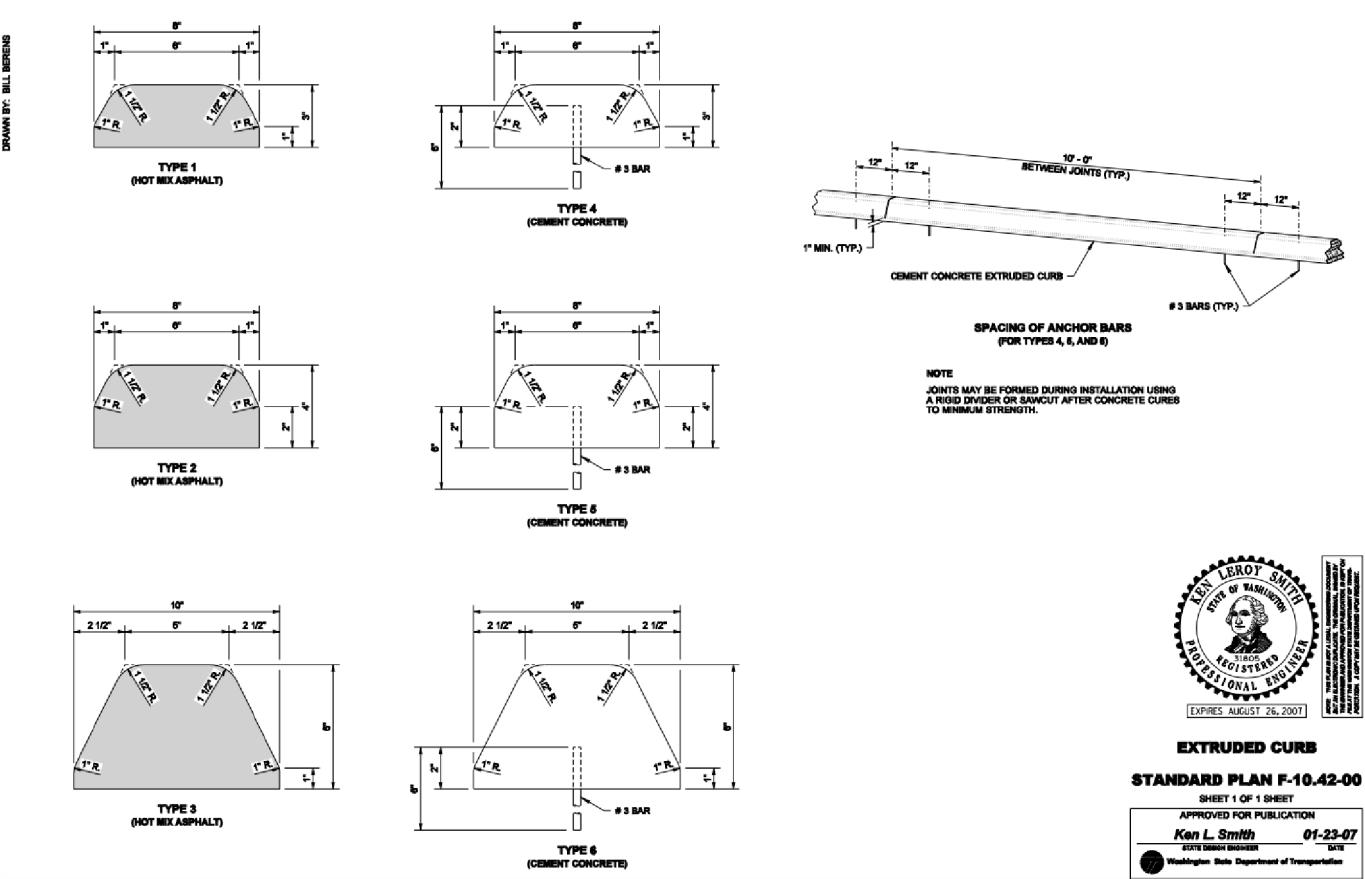
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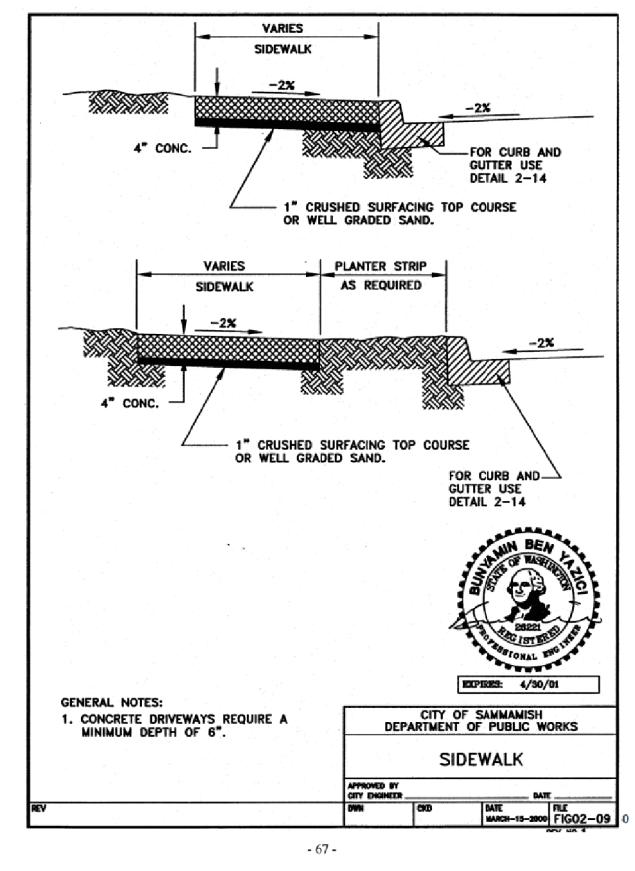
DA 06. 07. 11.

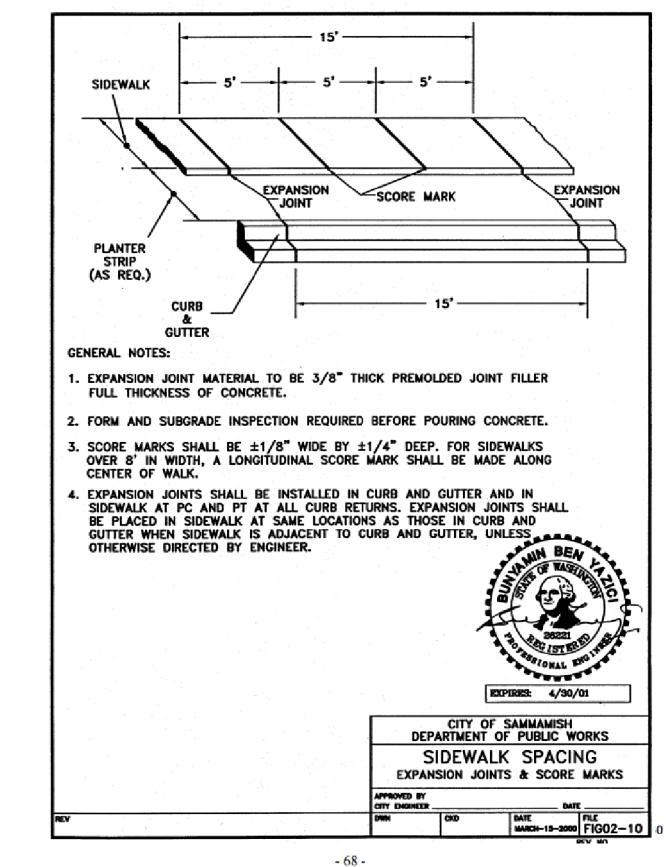
DRAFTED BY: CEN DESIGNED BY: YLP PROJECT ENGINEER: MAJ *DATE:* **02.15.17** PROJECT NO.: **15065** 

DRAWING: C15 SHEET: 15 OF 31

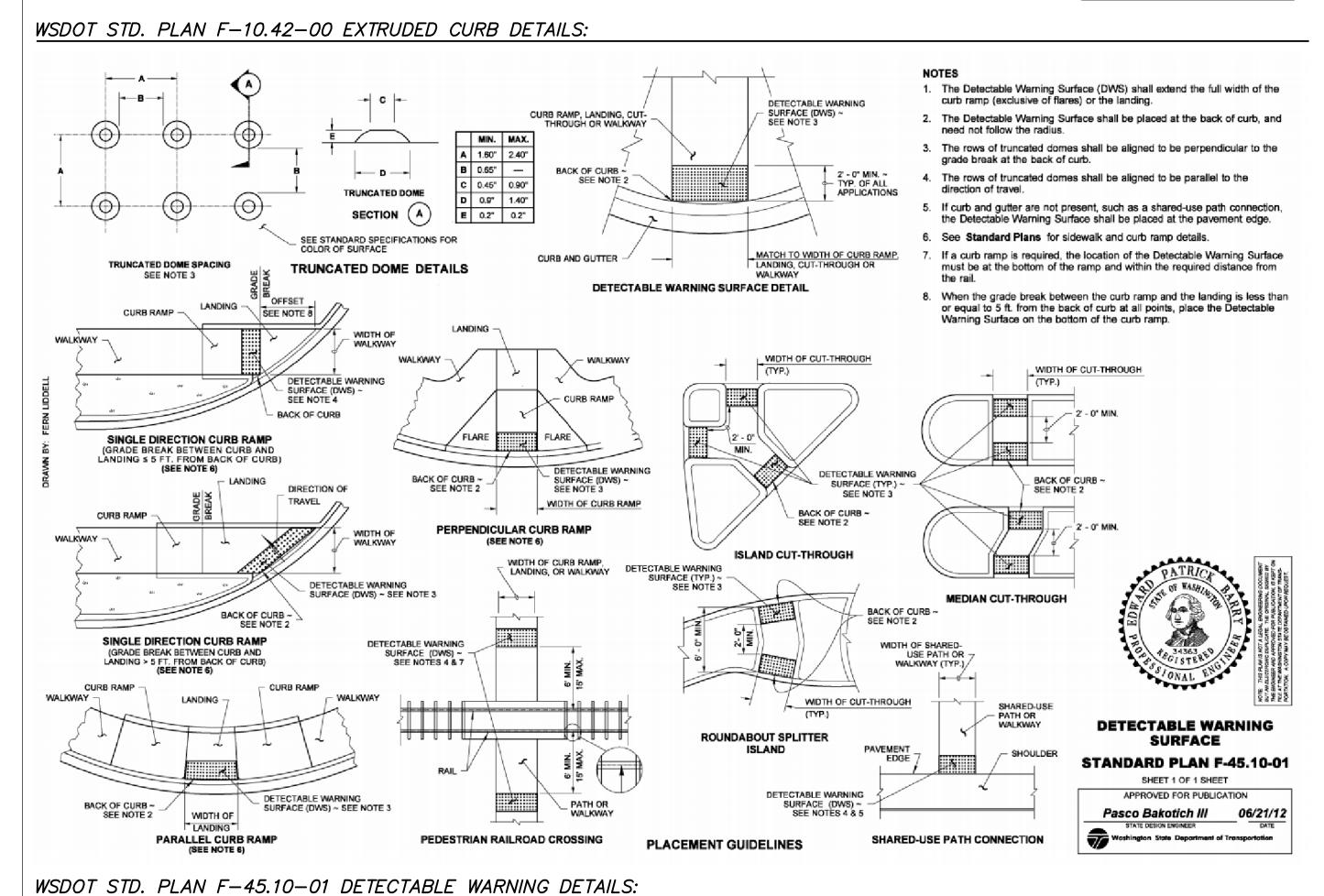
## PENNY LANE SOUTH

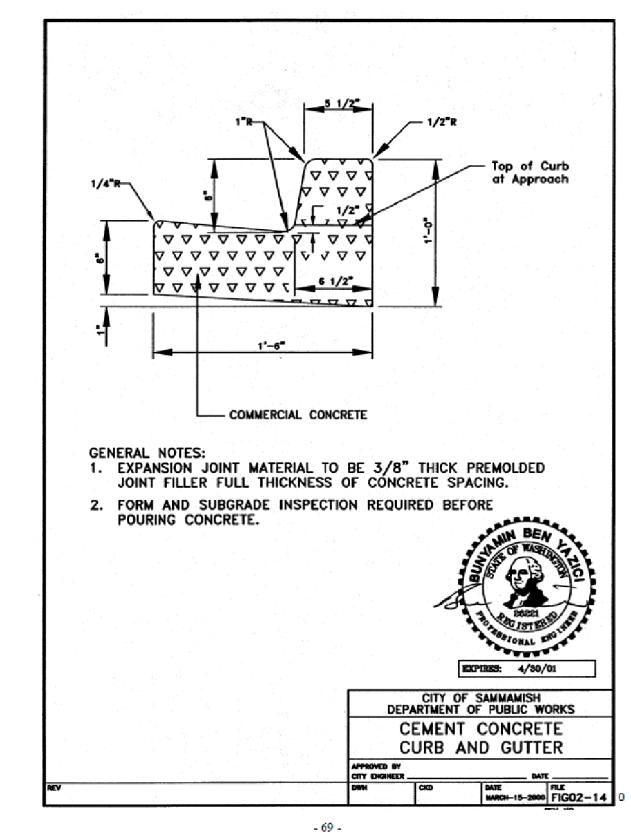






CITY OF SAMMAMISH FIGO2-09 & FIGO2-10 SIDEWALK DETAILS:





CITY OF SAMMAMISH FIGO2-14 CURB DETAILS:

Examined and Approved per SMC 20.05 for SDP2017-00575 this\_\_\_\_day of\_\_ City Planner Public Works Development Review Engineer

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D.R. STRONG

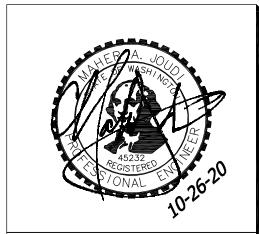
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SUBDIVISION

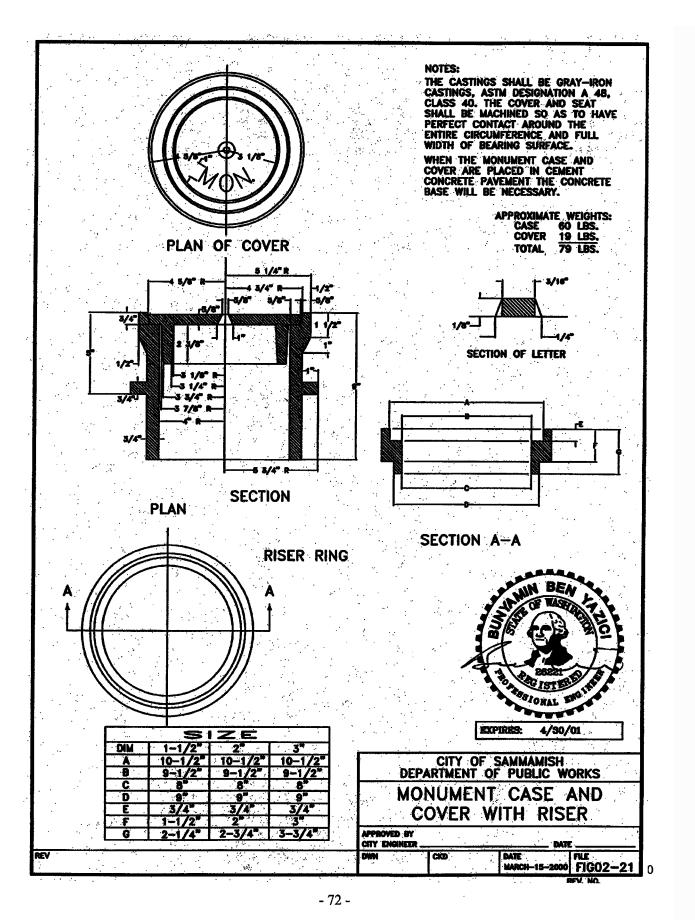
City of Sammamish Approval

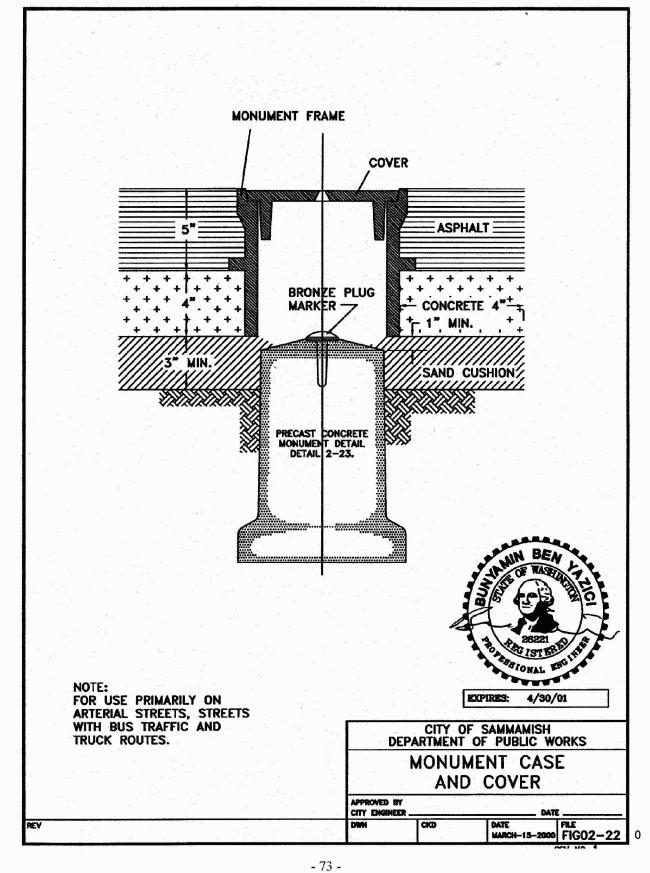
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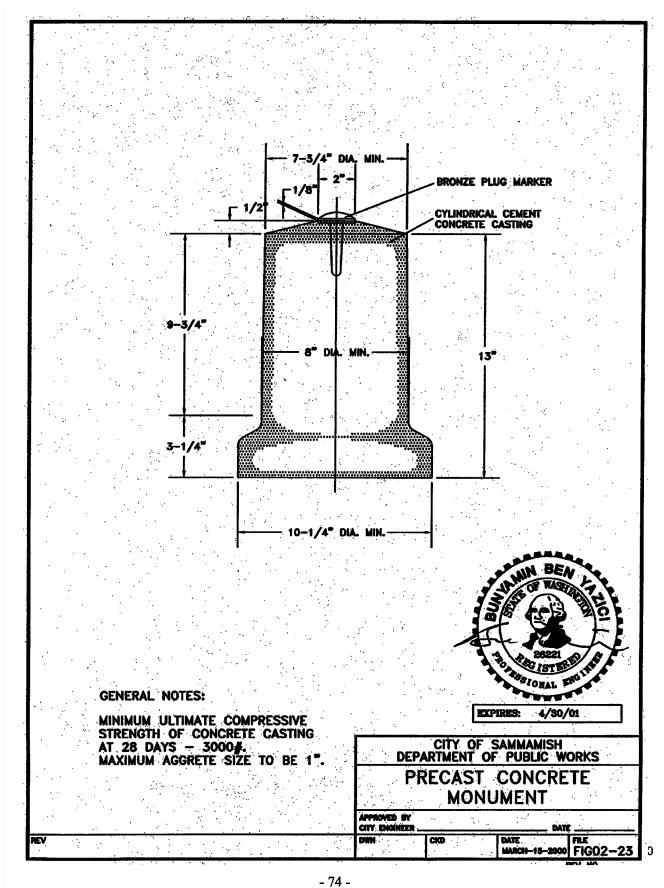
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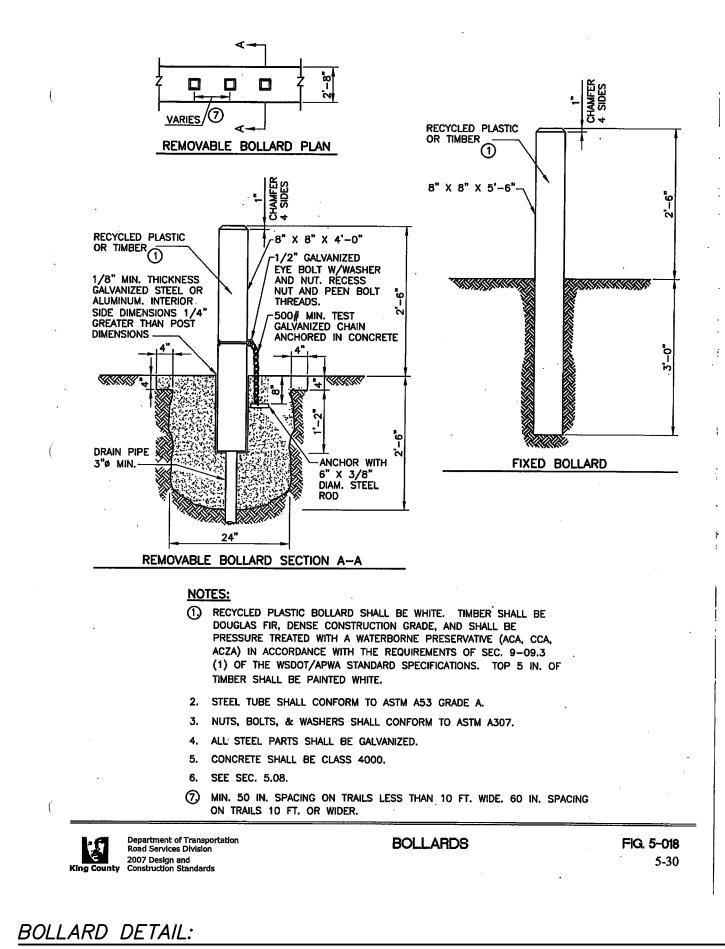
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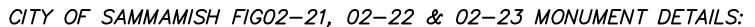
## PENNY LANE SOUTH

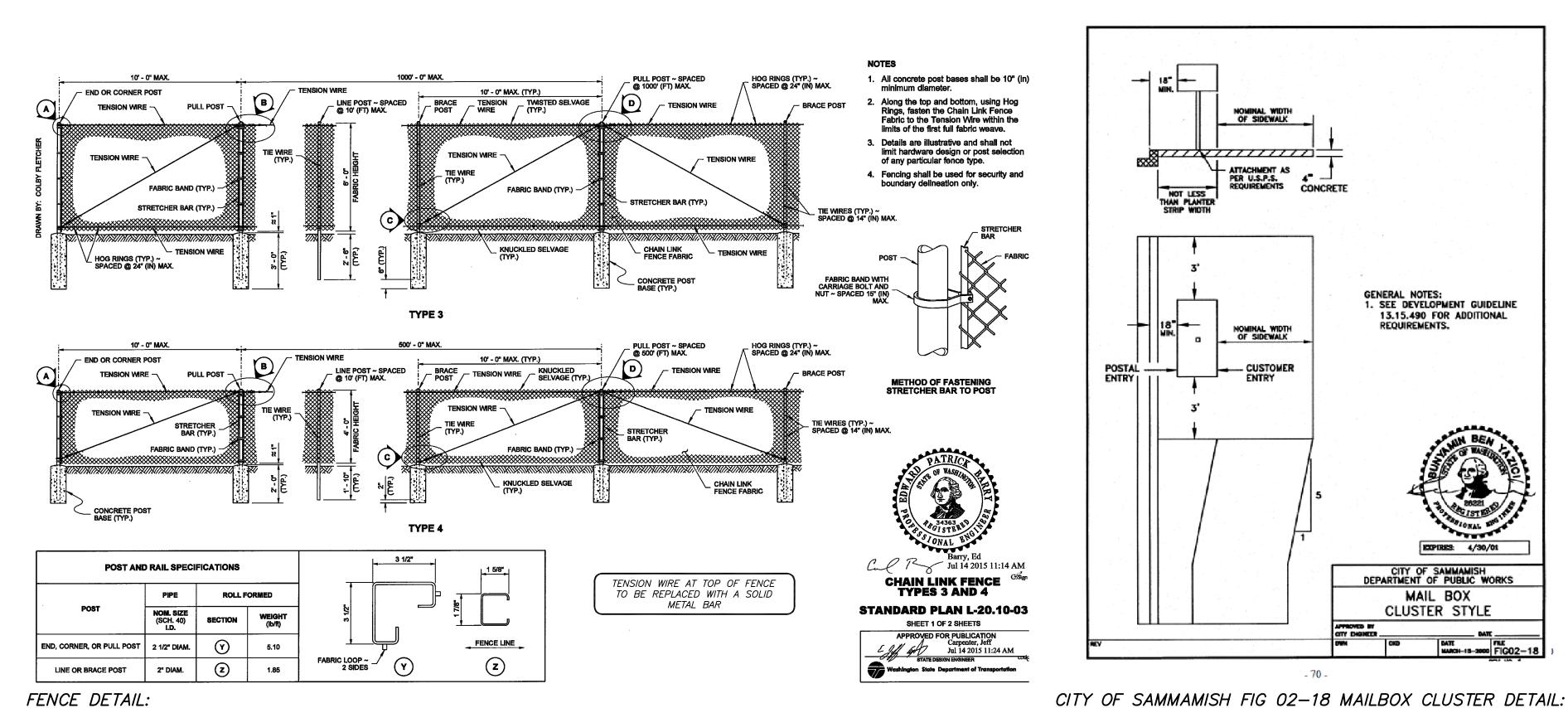


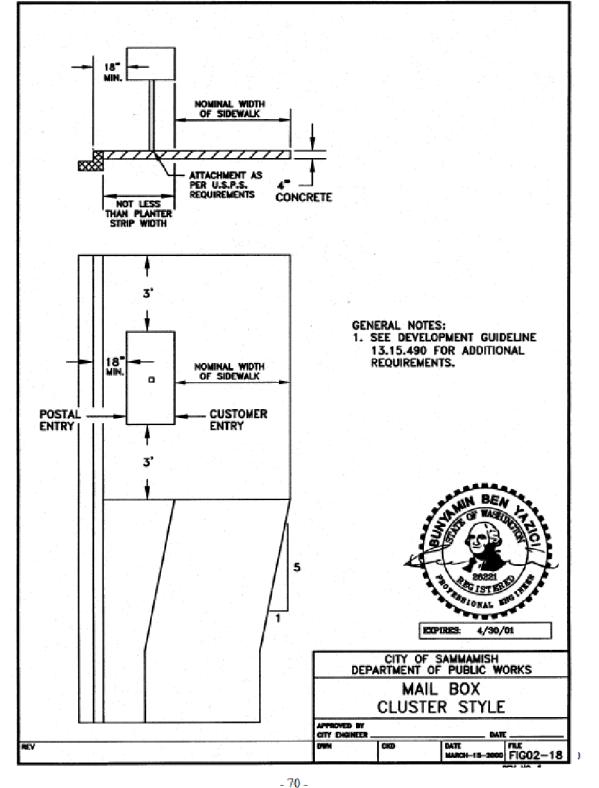


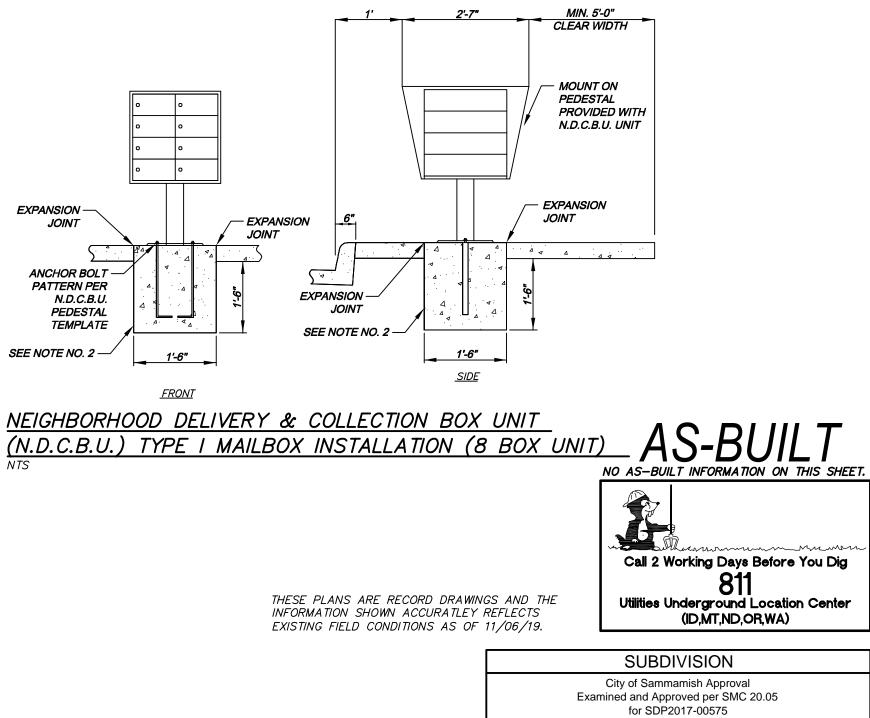


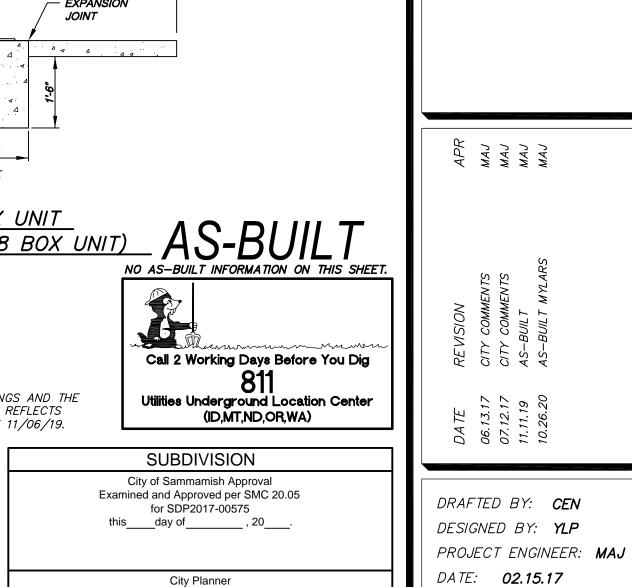












Public Works Development Review Engineer

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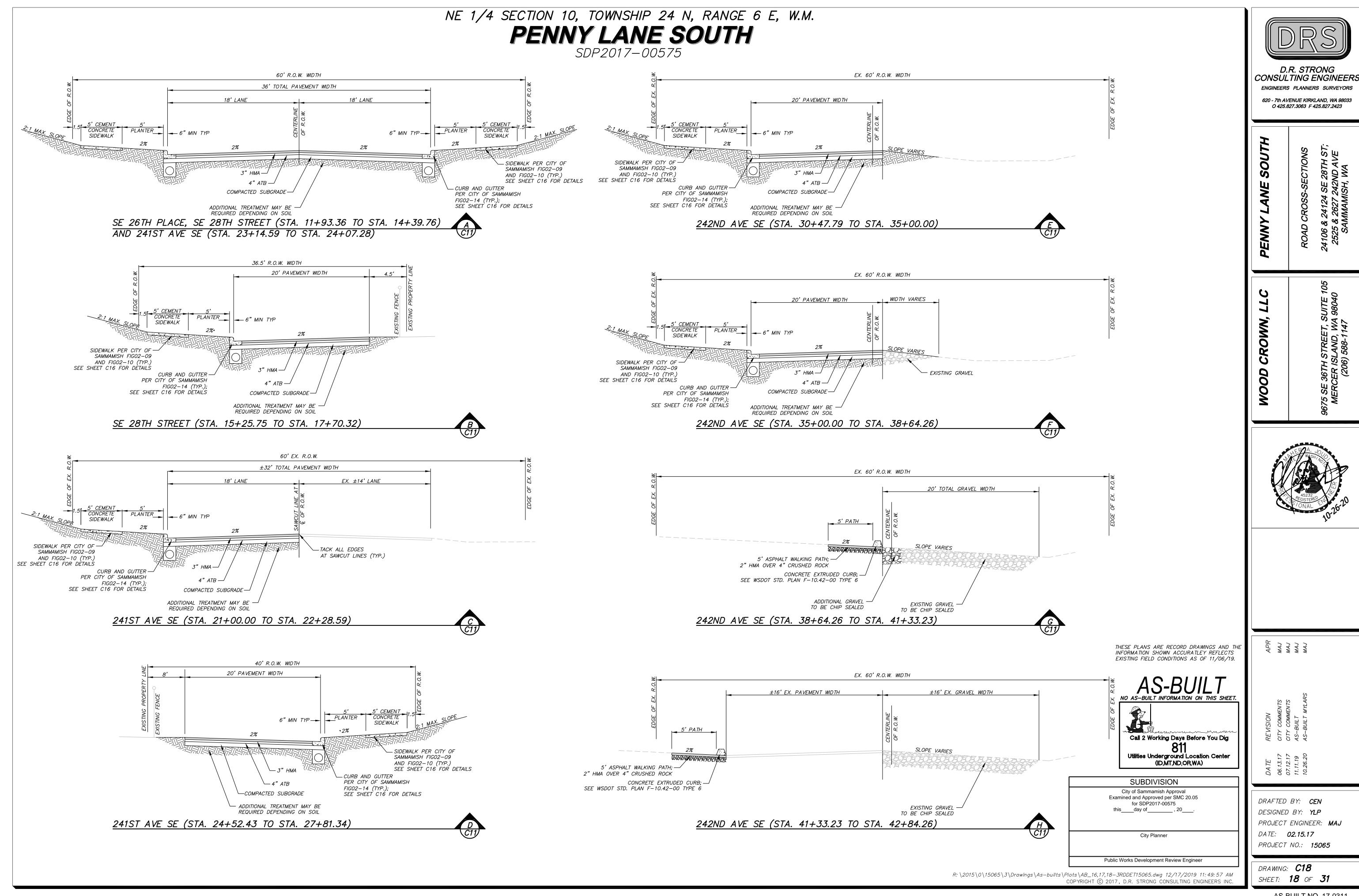
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PROJECT NO.: **15065** 

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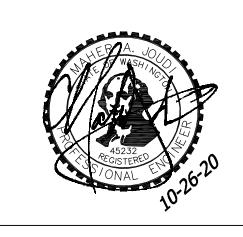


# PENNY LANE SOUTH SDP2017-00575



D.R. STRONG CONSULTING ENGINEERS ENGINEERS PLANNERS SURVEYORS

620 - 7th AVENUE KIRKLAND, WA 98033 O 425.827.3063 F 425.827.2423



APR MAS MAS MAS

Call 2 Working Days Before You Dig Utilities Underground Location Center (ID,MT,ND,OR,WA)

SUBDIVISION

City of Sammamish Approval Examined and Approved per SMC 20.05

for SDP2017-00575

City Planner

Public Works Development Review Engineer

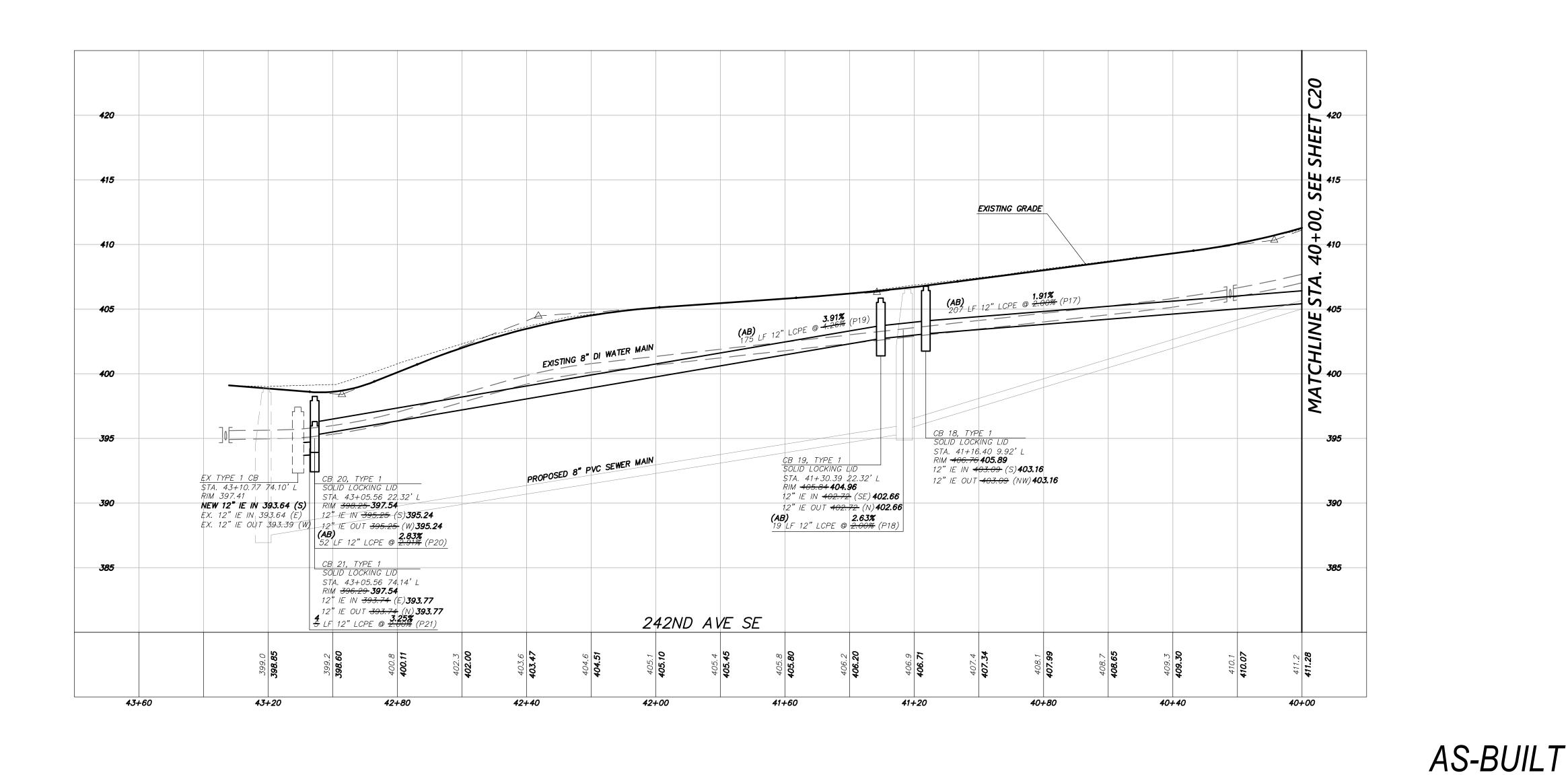
this\_\_\_\_day of\_\_\_\_\_\_, 20\_\_

DATE 06.13.17 07.12.17 11.11.19 10.26.20

DRAFTED BY: CEN DESIGNED BY: YLP

PROJECT ENGINEER: MAJ DATE: **02.15.17** PROJECT NO.: **15065** 

DRAWING: C19 SHEET: 19 OF 31



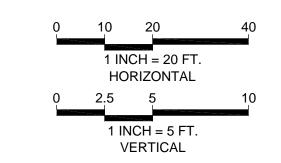
### NOTES:

1. ALL CATCH BASINS ARE VANED GRATES UNLESS OTHERWISE NOTED.

2. ALL OFFSETS ARE TO CENTER OF STRUCTURE.

3. BACKFILL OVER PIPES AND UTILITIES IN RIGHT-OF-WAY SHALL BE 5%" MINUS CRUSHED ROCK.

### THESE PLANS ARE RECORD DRAWINGS AND THE INFORMATION SHOWN ACCURATLEY REFLECTS EXISTING FIELD CONDITIONS AS OF 11/06/19.

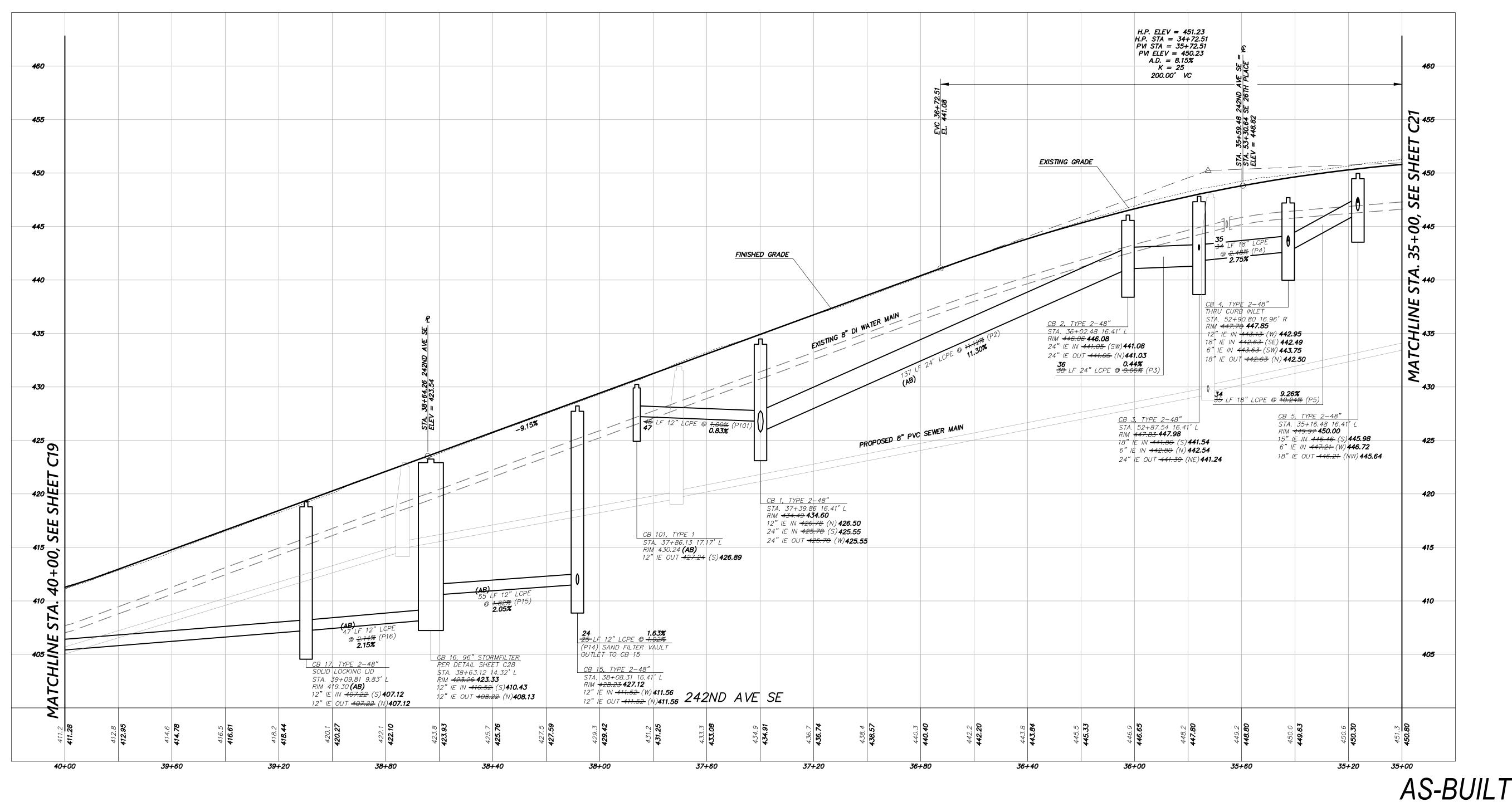


## SEE SHEET C14 FOR STORM DRAINAGE PLAN VIEW

VERTICAL DATUM: NAVD 88, PER KING COUNTY VERTICAL CONTROL HORIZONTAL DATUM: NAD 1983/91 UTILITY CROSSING NOTE:

(X) SEE SHEET C14 FOR UTILITY CROSSING TABLE R:  $\2015\0\15065\3\Drawings\As-builts\Plots\AB_19,20,21-3RDSDPR15065.dwg\ 10/26/2020\ 5:10:40\ PM$  COPYRIGHT © 2017, D.R. STRONG CONSULTING ENGINEERS INC.

# PENNY LANE SOUTH SDP2017-00575

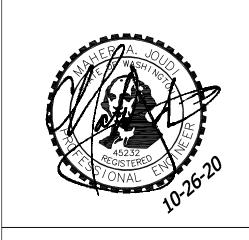




D.R. STRONG CONSULTING ENGINEERS ENGINEERS PLANNERS SURVEYORS

620 - 7th AVENUE KIRKLAND, WA 98033 O 425.827.3063 F 425.827.2423

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TE 13.1 12.1 11.1 DA 06.1 07.1 11.1

Call 2 Working Days Before You Dig

Utilities Underground Location Center

(ID,MT,ND,OR,WA)

SUBDIVISION

City of Sammamish Approval Examined and Approved per SMC 20.05

for SDP2017-00575

City Planner

Public Works Development Review Engineer

this\_\_\_\_day of\_\_\_\_

DRAFTED BY: CEN

DESIGNED BY: YLP PROJECT ENGINEER: MAJ DATE: **02.15.17** PROJECT NO.: **15065** 

DRAWING: C20 SHEET: **20** OF **31** 

NOTES: 1. ALL CATCH BASINS ARE VANED GRATES UNLESS OTHERWISE NOTED.

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3. BACKFILL OVER PIPES AND UTILITIES IN RIGHT-OF-WAY SHALL BE 5/8" MINUS CRUSHED ROCK.

## SEE SHEET C14 FOR STORM DRAINAGE PLAN VIEW

THESE PLANS ARE RECORD DRAWINGS AND THE

INFORMATION SHOWN ACCURATLEY REFLECTS

EXISTING FIELD CONDITIONS AS OF 11/06/19.

VERTICAL DATUM: NAVD 88, PER KING COUNTY VERTICAL CONTROL HORIZONTAL DATUM: NAD 1983/91

UTILITY CROSSING NOTE: (X) SEE SHEET C14 FOR UTILITY CROSSING TABLE

R:  $\2015\0\15065\3\Drawings\As-builts\Plots\AB_19,20,21-3RDSDPR15065.dwg\ 10/26/2020\ 5:10:40\ PM$  COPYRIGHT © 2017, D.R. STRONG CONSULTING ENGINEERS INC.

1 INCH = 20 FT.

HORIZONTAL

1 INCH = 5 FT.

VERTICAL

2.5 5

# PENNY LANE SOUTH SDP2017-00575



CONSULTING ENGINEERS ENGINEERS PLANNERS SURVEYORS

620 - 7th AVENUE KIRKLAND, WA 98033 O 425.827.3063 F 425.827.2423

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DRAFTED BY: **CEN** 

Call 2 Working Days Before You Dig

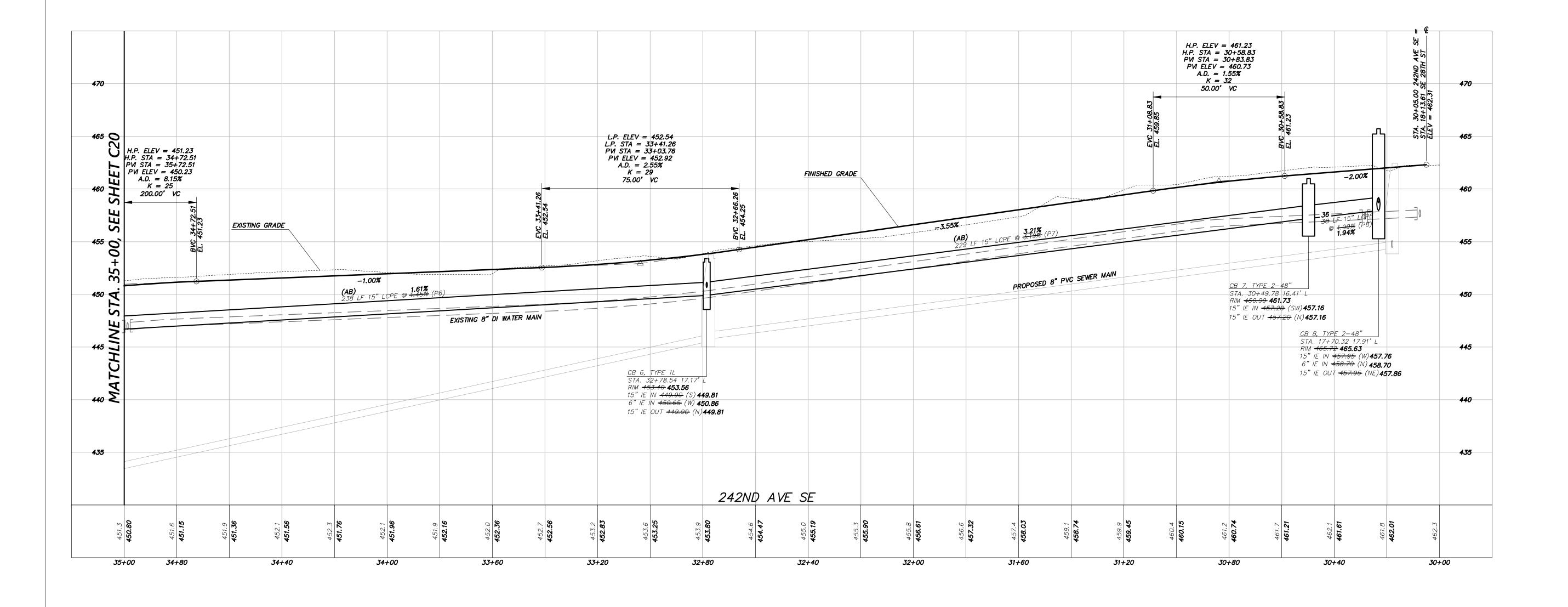
AS-BUILT

Utilities Underground Location Center (ID,MT,ND,OR,WA)

SUBDIVISION City of Sammamish Approval Examined and Approved per SMC 20.05 for SDP2017-00575 this\_\_\_day of\_\_\_\_, 20\_\_ City Planner

DESIGNED BY: YLP PROJECT ENGINEER: MAJ DATE: **02.15.17** PROJECT NO.: **15065** 

DRAWING: C21 SHEET: **21** OF **31** 



NOTES:

CRUSHED ROCK.

1. ALL CATCH BASINS ARE VANED GRATES UNLESS OTHERWISE NOTED.

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### 1 INCH = 20 FT. HORIZONTAL 2.5 5 1 INCH = 5 FT. VERTICAL

## SEE SHEET C14 FOR STORM DRAINAGE PLAN VIEW

VERTICAL DATUM: NAVD 88, PER KING COUNTY VERTICAL CONTROL HORIZONTAL DATUM: NAD 1983/91

UTILITY CROSSING NOTE: (X) SEE SHEET C14 FOR UTILITY CROSSING TABLE

R:  $\2015\0\15065\3\D$ rawings $\As-builts\Plots\AB_19,20,21-3$ RDSDPR15065.dwg 10/26/2020 5:10:40 PM COPYRIGHT © 2017, D.R. STRONG CONSULTING ENGINEERS INC.

### Public Works Development Review Engineer

### AS-BUILT NO. 17-0314

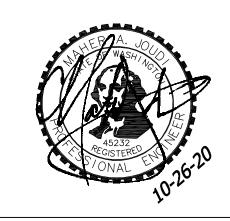
# PENNY LANE SOUTH SDP2017-00575



620 - 7th AVENUE KIRKLAND, WA 98033 O 425.827.3063 F 425.827.2423

ENGINEERS PLANNERS SURVEYORS

CROWN,



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Call 2 Working Days Before You Dig Vorking Days 2811
Utilities Underground Location Center (ID,MT,ND,OR,WA)

AS-BUILT

SUBDIVISION

City of Sammamish Approval Examined and Approved per SMC 20.05 for SDP2017-00575 this\_\_\_\_day of\_\_\_\_

City Planner

Public Works Development Review Engineer

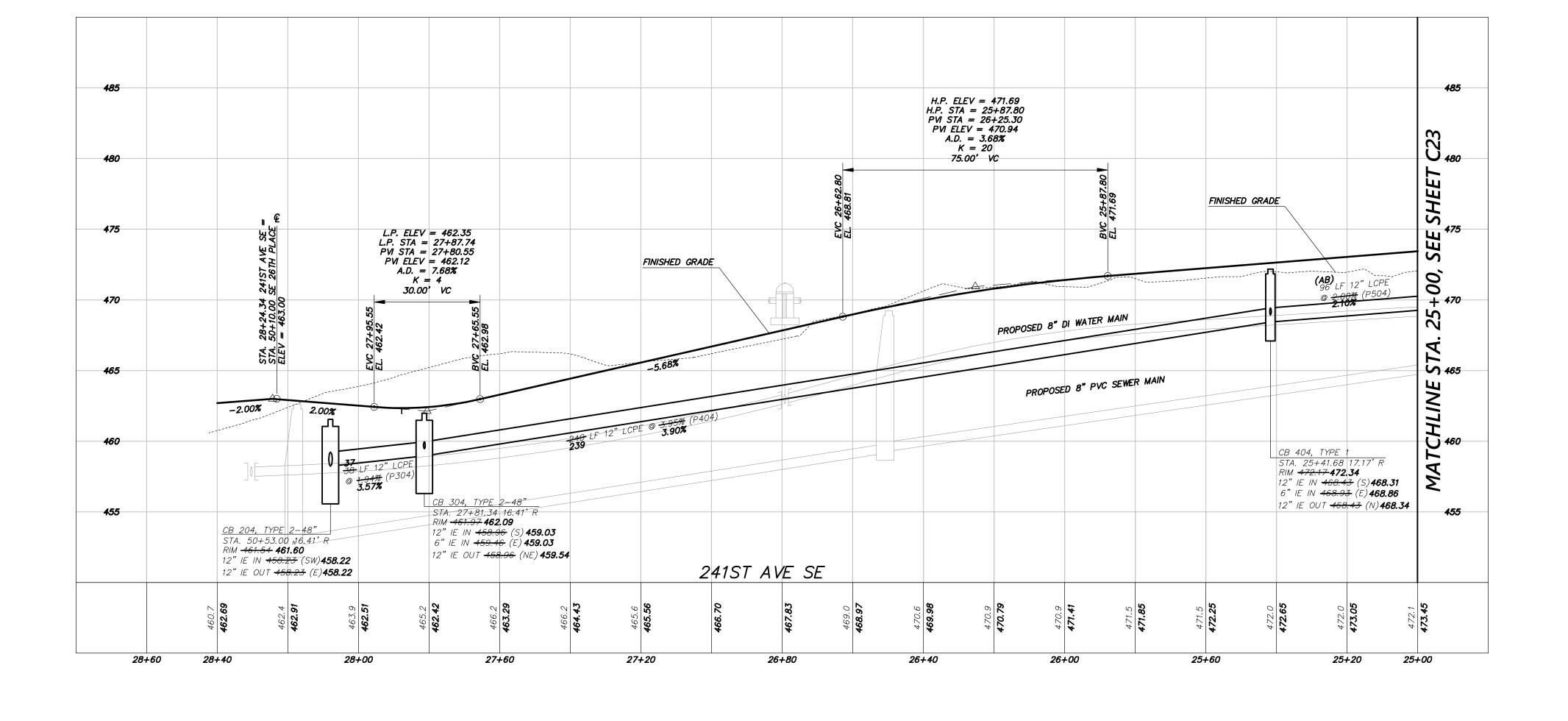
PROJECT ENGINEER: MAJ DATE: **02.15.17** PROJECT NO.: **15065** 

> DRAWING: C22 SHEET: **22** OF **31**

DATE 06.13.17 07.12.17 11.11.19 10.26.20

DRAFTED BY: **CEN** 

DESIGNED BY: YLP



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## SEE SHEET C14 FOR STORM DRAINAGE PLAN VIEW

VERTICAL DATUM: NAVD 88, PER KING COUNTY VERTICAL CONTROL HORIZONTAL DATUM: NAD 1983/91

UTILITY CROSSING NOTE: (X) SEE SHEET C14 FOR UTILITY CROSSING TABLE

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# PENNY LANE SOUTH SDP2017-00575

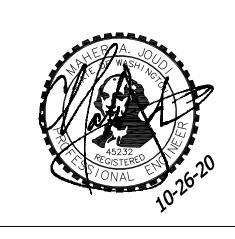


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620 - 7th AVENUE KIRKLAND, WA 98033 O 425.827.3063 F 425.827.2423

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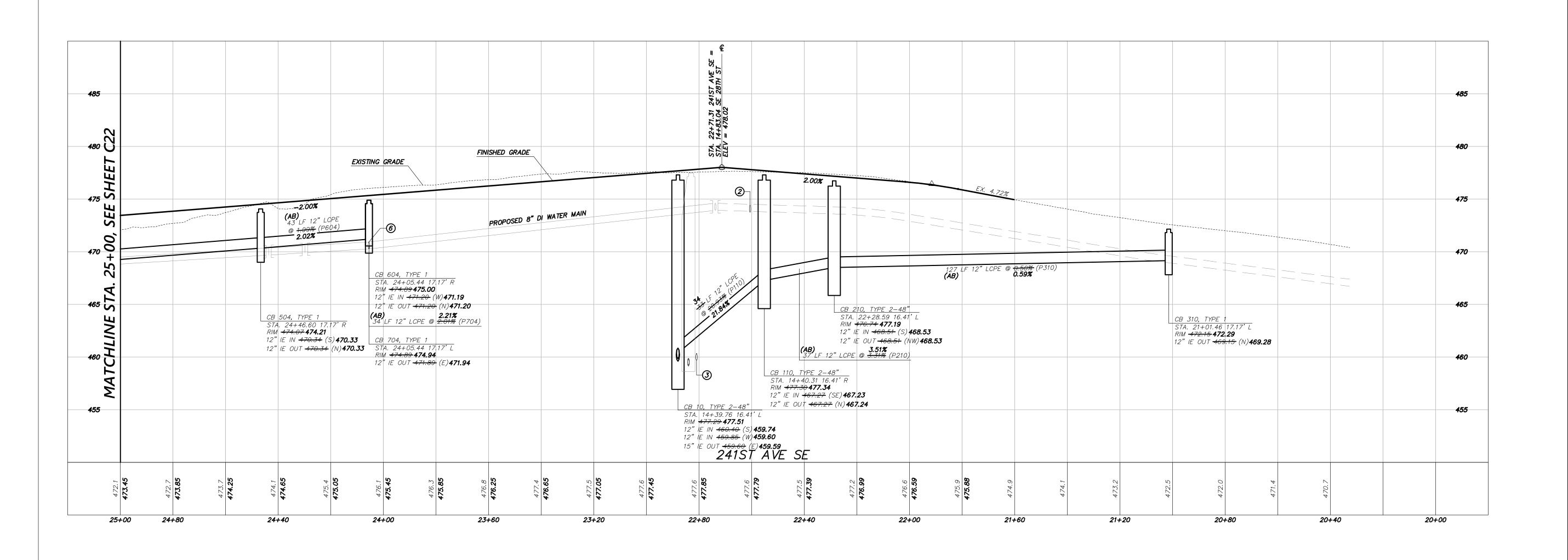
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DATE 06.13.17 07.12.17 11.11.19 10.26.20

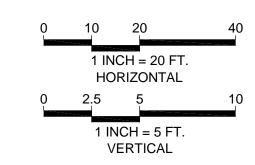
DRAFTED BY: **CEN** DESIGNED BY: YLP PROJECT ENGINEER: MAJ DATE: **02.15.17** 

PROJECT NO.: **15065** DRAWING: C23

SHEET: **23** OF **31** 



THESE PLANS ARE RECORD DRAWINGS AND THE INFORMATION SHOWN ACCURATLEY REFLECTS EXISTING FIELD CONDITIONS AS OF 11/06/19.



# Vorking Days - 811 Utilities Underground Location Center (ID,MT,ND,OR,WA) Call 2 Working Days Before You Dig

AS-BUILT

SUBDIVISION

City of Sammamish Approval Examined and Approved per SMC 20.05

for SDP2017-00575

City Planner

Public Works Development Review Engineer

this\_\_\_\_day of\_\_\_\_\_, 20\_\_

## SEE SHEET C14 FOR STORM DRAINAGE PLAN VIEW

VERTICAL DATUM: NAVD 88, PER KING COUNTY VERTICAL CONTROL HORIZONTAL DATUM: NAD 1983/91

UTILITY CROSSING NOTE:

(X) SEE SHEET C14 FOR UTILITY CROSSING TABLE

R:  $\2015\0\15065\3\Drawings\As-builts\Plots\AB\_22,23-3RDSDPR15065.dwg\ 12/17/2019\ 11:52:41\ AM$  COPYRIGHT © 2017, D.R. STRONG CONSULTING ENGINEERS INC.

## NOTES:

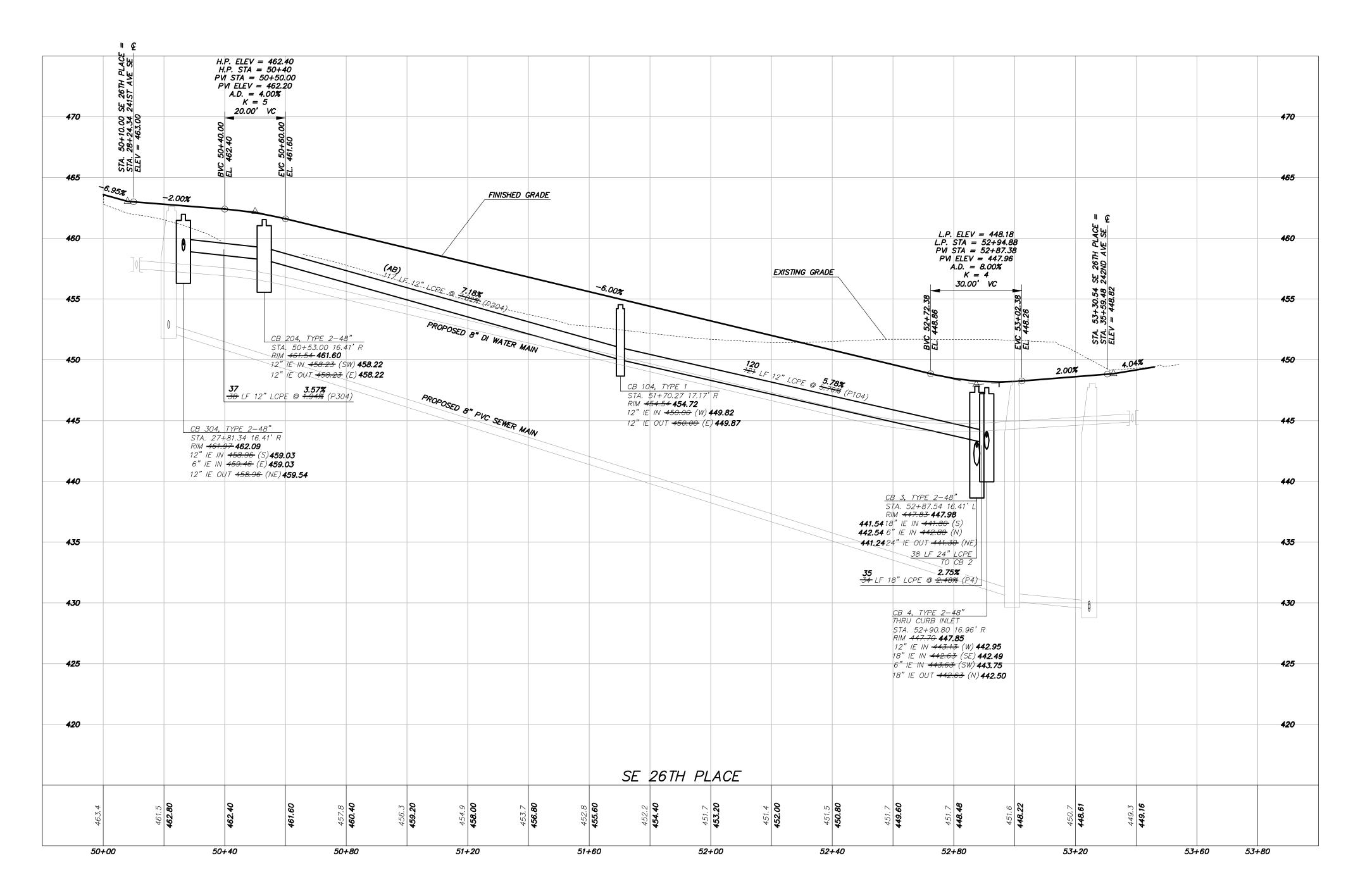
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## PENNY LANE SOUTH

SDP2017-00575





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CONSULTING ENGINEERS
ENGINEERS PLANNERS SURVEYORS

620 - 7th AVENUE KIRKLAND, WA 98033 O 425.827.3063 F 425.827.2423

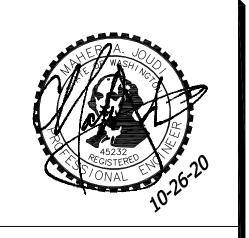
ANE SOUTH

CROWN,

SE 26TH PLACE D AND STORM PROFIL 06 & 24124 SE 28TH S

SE ROAD AN 24106 & 2 2525 &

SE 36TH STREET, SUITE 10



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AS-BUILT

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Utilities Underground Location Center

(ID,MT,ND,OR,WA)

SUBDIVISION

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for SDP2017-00575

City Planner

Public Works Development Review Engineer

this\_\_\_\_day of\_\_\_\_\_, 20\_

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26.20 AS-BUIL

DATE 06.13.17 07.12.17 11.11.19

DRAFTED BY: CEN

DESIGNED BY: YLP

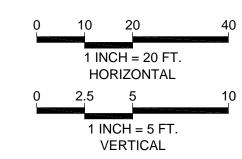
PROJECT ENGINEER: MAJ

DATE: 02.15.17

PROJECT NO.: 15065

DRAWING: **C24**SHEET: **24** OF **31** 

THESE PLANS ARE RECORD DRAWINGS AND THE INFORMATION SHOWN ACCURATLEY REFLECTS EXISTING FIELD CONDITIONS AS OF 11/06/19.



## SEE SHEET C14 FOR STORM DRAINAGE PLAN VIEW

VERTICAL DATUM:

NAVD 88, PER KING COUNTY VERTICAL CONTROL

HORIZONTAL DATUM:

NAD 1983/91

## UTILITY CROSSING NOTE:

X SEE SHEET C14 FOR UTILITY CROSSING TABLE

R:  $\2015\0\15065\3\Drawings\As-builts\Plots\AB\_24-3RDSDPR15065.dwg$  12/17/2019 11:53:27 AM COPYRIGHT © 2017, D.R. STRONG CONSULTING ENGINEERS INC.

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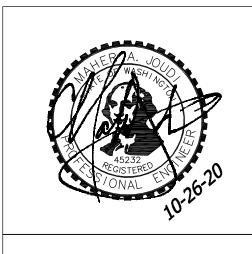
# PENNY LANE SOUTH SDP2017-00575



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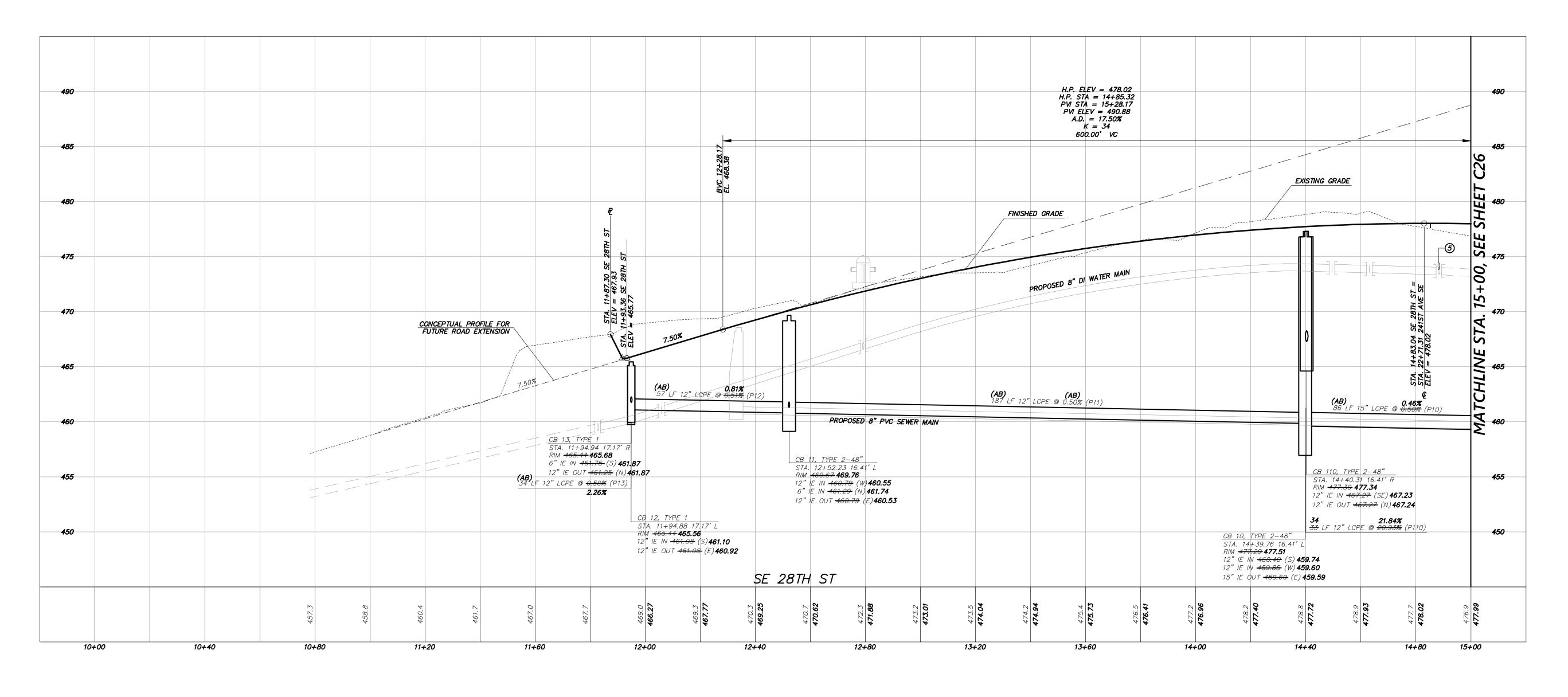


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DATE 06.13.17 07.12.17 11.11.19 10.26.20

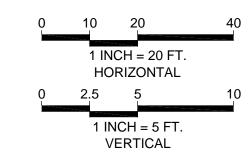
DRAFTED BY: **CEN** DESIGNED BY: YLP PROJECT ENGINEER: MAJ DATE: **02.15.17** PROJECT NO.: **15065** 

DRAWING: C25 SHEET: **25** OF **31** 



# AS-BUILT

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SUBDIVISION

City of Sammamish Approval Examined and Approved per SMC 20.05

for SDP2017-00575

City Planner

Public Works Development Review Engineer

this\_\_\_\_day of\_\_\_\_\_, 20\_\_

## SEE SHEET C14 FOR STORM DRAINAGE PLAN VIEW

VERTICAL DATUM: NAVD 88, PER KING COUNTY VERTICAL CONTROL HORIZONTAL DATUM: NAD 1983/91

UTILITY CROSSING NOTE:

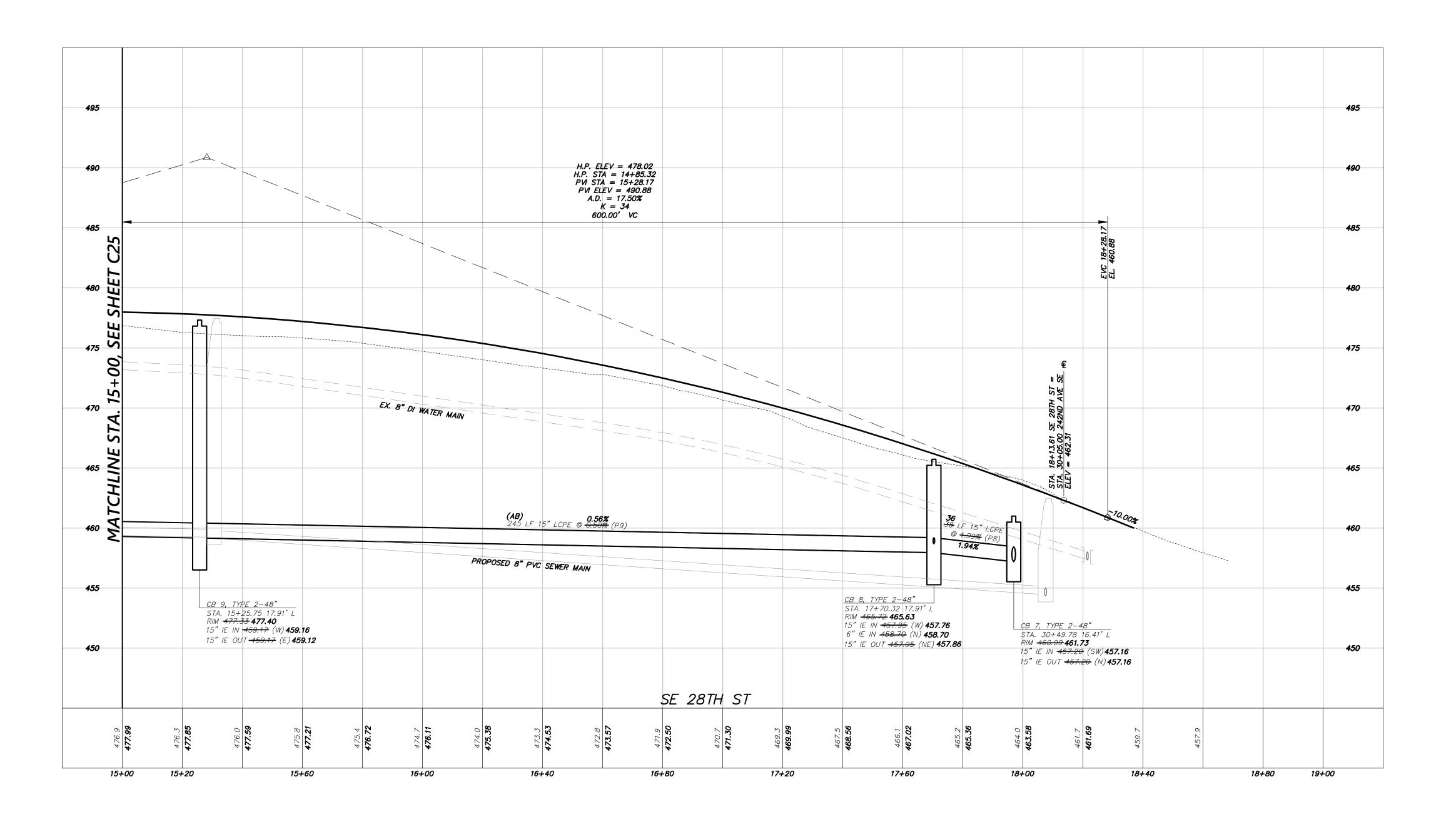
(X) SEE SHEET C14 FOR UTILITY CROSSING TABLE

R: \2015\0\15065\3\Drawings\As-builts\Plots\AB\_25,26-3RDSDPR15065.dwg 12/17/2019 11:54:42 AM COPYRIGHT © 2017, D.R. STRONG CONSULTING ENGINEERS INC.

### NOTES:

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- 3. BACKFILL OVER PIPES AND UTILITIES IN RIGHT-OF-WAY SHALL BE 5/8" MINUS CRUSHED ROCK.

# PENNY LANE SOUTH SDP2017-00575



NOTES:

CRUSHED ROCK.

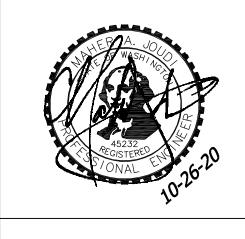
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DRAFTED BY: **CEN** DESIGNED BY: YLP PROJECT ENGINEER: MAJ DATE: **02.15.17** PROJECT NO.: **15065** 

DRAWING: **C26** SHEET: **26** OF **31** 

AS-BUILT

SUBDIVISION

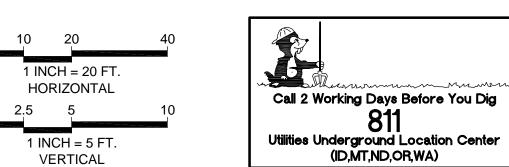
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Public Works Development Review Engineer

this\_\_\_\_day of\_\_\_\_\_, 20\_\_



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VERTICAL DATUM: NAVD 88, PER KING COUNTY VERTICAL CONTROL HORIZONTAL DATUM: NAD 1983/91

## UTILITY CROSSING NOTE:

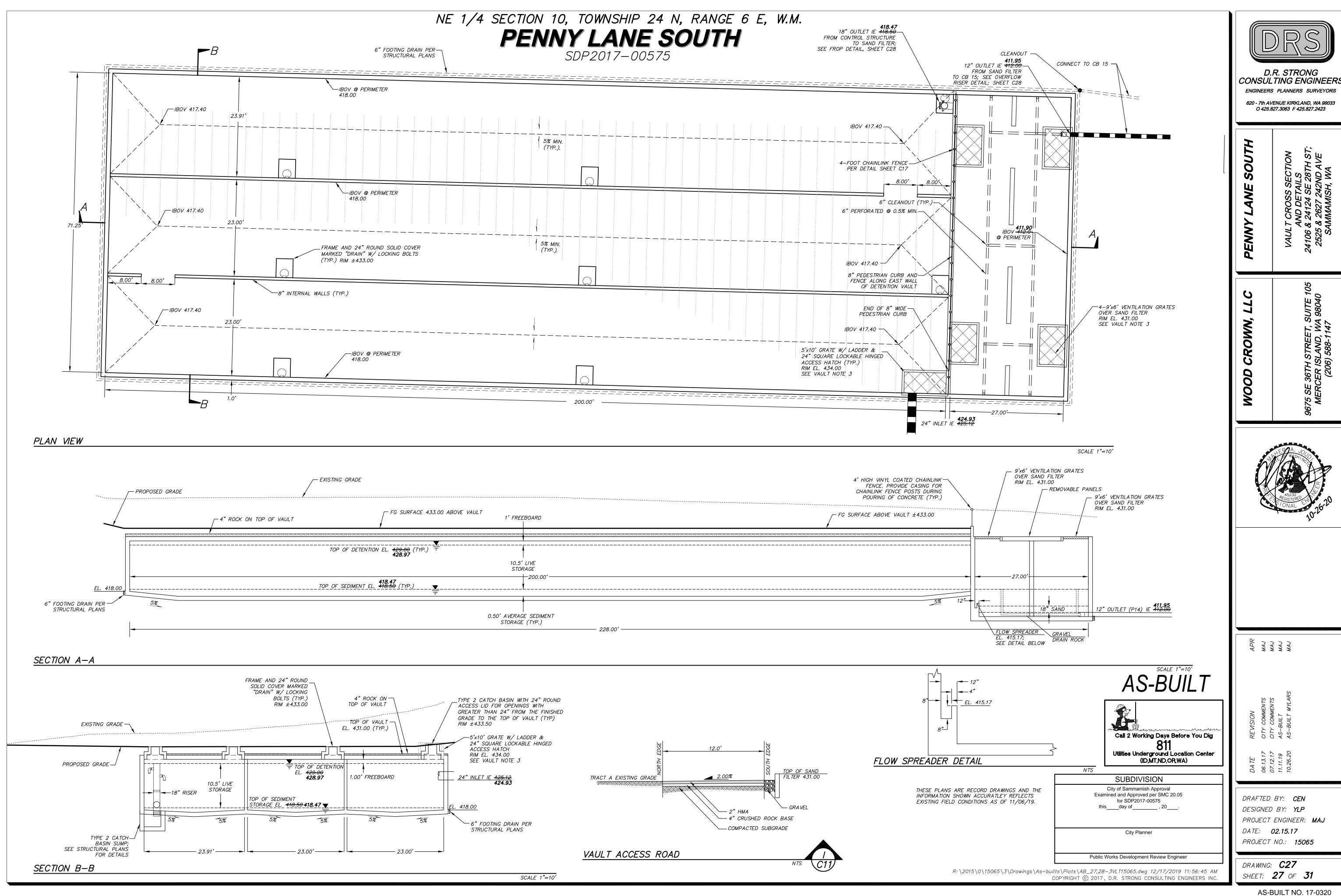
X) SEE SHEET C14 FOR UTILITY CROSSING TABLE

THESE PLANS ARE RECORD DRAWINGS AND THE

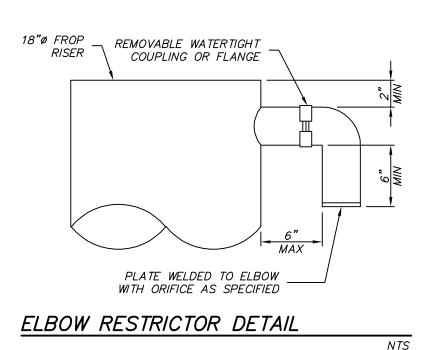
INFORMATION SHOWN ACCURATLEY REFLECTS

EXISTING FIELD CONDITIONS AS OF 11/06/19.

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## PENNY LANE SOUTH



### **VAULT NOTES:**

1. ACCESS OPENINGS SHALL BE POSITIONED A MAX. OF 50 FT FROM ANY LOCATION WITHIN THE VAULT, AND AT THE INLET AND OUTLET. 2. IBOV = INSIDE BOTTOM OF VAULT ELEVATION.

### FROP NOTES:

1. OUTLET CAPACITY: 100-YR PEAK FLOW

2. METAL PARTS: CORROSION RESISTANT. NON-GALVANIZED PARTS PREFERRED. GALVANIZED PIPE PARTS TO

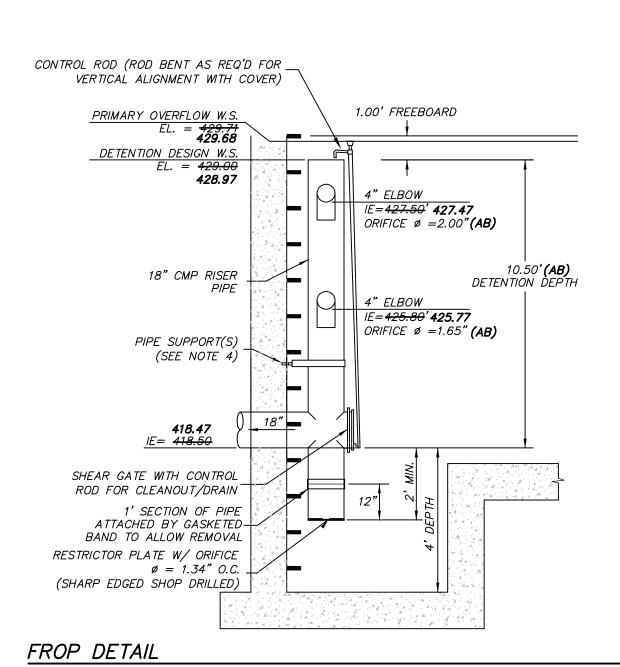
3. IF METAL OUTLET PIPE CONNECTS TO CEMENT CONCRETE PIPE: OUTLET PIPE TO HAVE SMOOTH O.D. EQUAL TO CONCRETE PIPE I.D. LESS 1/4"

4. PROVIDE AT LEAST ONE 3"X.090 GAUGE SUPPORT BRACKET ANCHORED TO CONCRETE WALL. (MAXIMUM 3'

5. LOCATE ELBOW RESTRICTOR(S) AS NECESSARY TO PROVIDE MINIMUM CLEARANCE AS SHOWN.

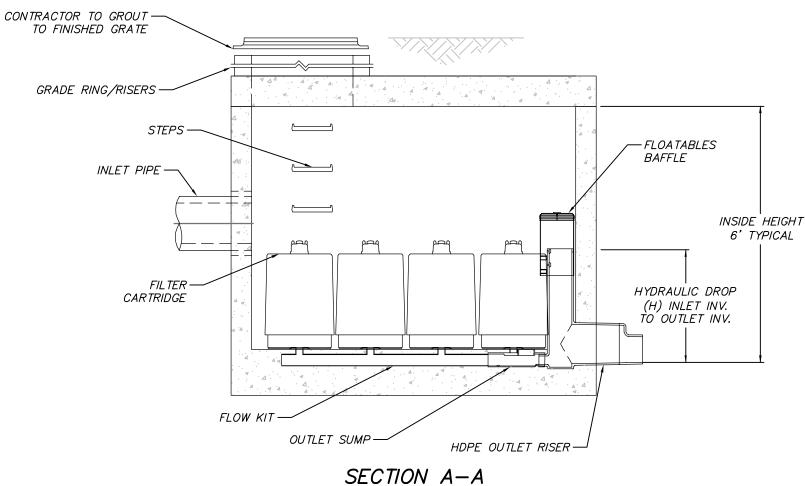
### DETENTION SUMMARY SHEET

STAGE	STAGE	LIVE ST	ORAGE VOLUI	ME (CF)
STORM	EL.	REQUIRED	PROVIDED	AS-BUILT
TOTAL	429.00	146,512	147,015	147,640
100 YEAR	429.32	150,949	151,496	152,140
25 YEAR	429.02	146,750	147,296	147,922
10 YEAR	429.00	146,576	147,015	147,640
2 YFAR	422.10	50.214	50.405	50.620



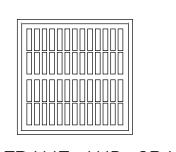
— OUTLET OUTLET A 96" I.D. MANHOLE TOP SLAB ACCESS -STRUCTURE SEE FRAME AND COVER DETAIL PLAN VIEW STANDARD OUTLET RISER

FLOWKIT: 43A



STORMFILTER DESIGN NOTES STORMFILTER TREATMENT CAPACITY IS A FUNCTION OF THE CARTRIDGE SELECTION AND THE NUMBER OF CARTRIDGES. THE STANDARD MANHOLE STYLE IS SHOWN WITH THE MAXIMUM NUMBER OF CARTRIDGES (14). VOLUME SYSTEM IS ALSO AVAILABLE WITH MAXIMUM 14 CARTRIDGES. Ø96" MANHOLE STORMFILTER PEAK HYDRAULIC CAPACITY IS 1.8 CFS. IF THE SITE CONDITIONS EXCEED 1.8 CFS AN UPSTREAM BYPASS STRUCTURE IS CARTRIDGE SELECTION RECOMMENDED HYDRAULIC DROP (I SPECIFIC FLOW RATE (gpm/sf) CARTRIDGE FLOW RATE (gpr

\* 1.67 gpm/sf SPECIFIC FLOW RATE IS APPROVED WITH PHOSPHOSORB ® (PSORB) MEDIA ONLY



30" FRAME AND GRATE (ALSO AVAILABLE IN ROUND)

STRUCTURE ID				16
WATER QUALITY	FLOW RAT	TE (cfs)		0.213
PEAK FLOW RAT	E (cfs)			0.575
RETURN PERIOD	OF PEAK I	FLOW (yrs)		25
CARTRIDGE HEI	GHT (27", 18	B", LOW DROP	(LD))	18"
NUMBER OF CAR	RTRIDGES I	REQUIRED		14
CARTRIDGE FLC	W RATE			7.5
MEDIA TYPE (PE	RLITE, ZPG	, PSORB)		CSF
PIPE DATA:	I.E.	MATERIAL	D	AMETER
INLET PIPE #1	<del>410.52-</del> 410	.43 LCPE		12"
INLET PIPE #2	*	*		*
OUTLET PIPE -	<del>408.22</del> 408	.13 LCPE		12"
RIM ELEVATION				<del>-423.26</del>
ANTI-FLOTATION	BALLAST	WIDTH		HEIGHT
NOTEO/ODEO/A	REQUIREM	* * *		*

GENERAL NOTES

1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.

2. DIMENSIONS MARKED WITH ( ) ARE REFERENCE DIMENSIONS. ACTUAL DIMENSIONS MAY VARY. 3. FOR SITE SPECIFIC DRAWINGS WITH DETAILED VAULT DIMENSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS

REPRESENTATIVE. www.ContechES.com 4. STORMFILTER WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS

5. STRUCTURE SHALL MEET AASHTO HS20 LOAD RATING, ASSUMING EARTH COVER OF 0' - 5' AND GROUNDWATER ELEVATION AT, OR BELOW, THE

OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 AND BE CAST WITH THE CONTECH LOGO.

FILTER CARTRIDGES SHALL BE MEDIA-FILLED, PASSIVE, SIPHON ACTUATED, RADIAL FLOW, AND SELF CLEANING. RADIAL MEDIA DEPTH SHALL BE 7-INCHES. FILTER MEDIA CONTACT TIME SHALL BE AT LEAST 38 SECONDS.
 SPECIFIC FLOW RATE IS EQUAL TO THE FILTER TREATMENT CAPACITY (gpm) DIVIDED BY THE FILTER CONTACT SURFACE AREA (sq ft).

A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE

SPECIFIED BY ENGINEER OF RECORD.

B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STORMFILTER STRUCTURE

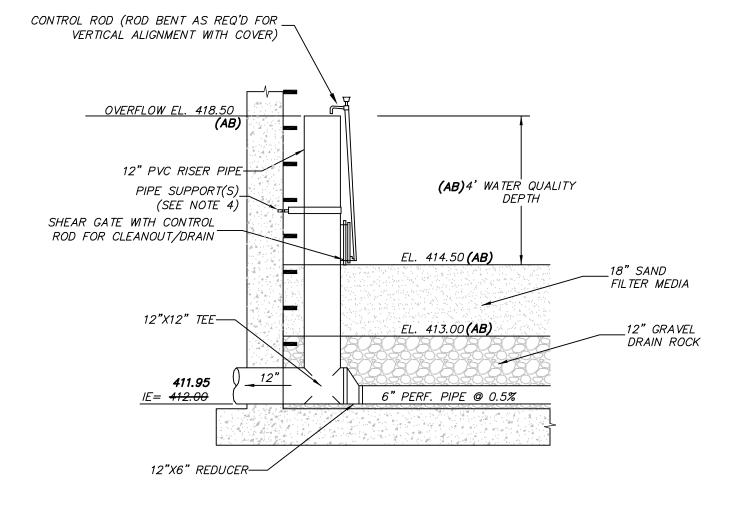
(LIFTING CLUTCHES PROVIDED). : CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS AND ASSEMBLE STRUCTURE

D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT INLET PIPE(S). E. CONTRACTOR TO PROVIDE AND INSTALL CONNECTOR TO THE OUTLET RISER STUB. STORMFILTER EQUIPPED WITH A DUAL DIAMETER HDPE

OUTLET STUB AND SAND COLLAR. IF OUTLET PIPE IS LARGER THAN 8 INCHES, CONTRACTOR TO REMOVE THE 8 INCH OUTLET STUB AT MOLDED

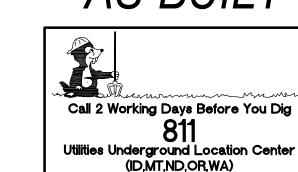
IN CUT LINE. COUPLING BY FERNCO OR EQUAL AND PROVIDED BY CONTRACTOR. F. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO PROTECT CARTRIDGES FROM CONSTRUCTION-RELATED EROSION RUNOFF.

STORMFILTER DETAIL



OVERFLOW RISER DETAIL

AS-BUILT



THESE PLANS ARE RECORD DRAWINGS AND THE INFORMATION SHOWN ACCURATLEY REFLECTS

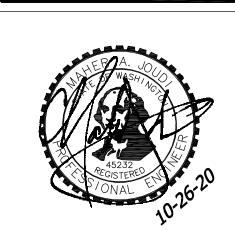
SUBDIVISION City of Sammamish Approval Examined and Approved per SMC 20.05 for SDP2017-00575 EXISTING FIELD CONDITIONS AS OF 11/06/19. this\_\_\_\_day of\_\_ City Planner Public Works Development Review Engineer

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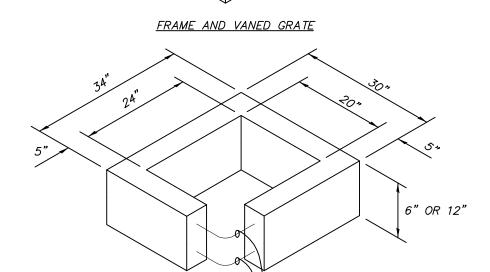
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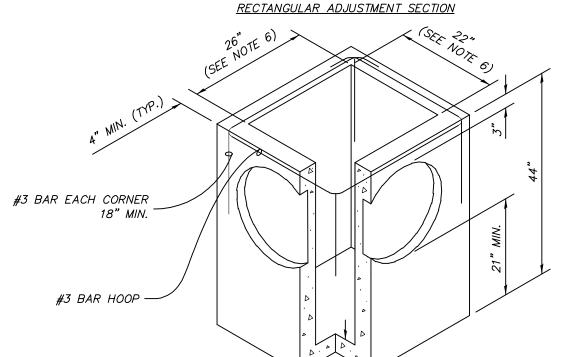
DRAFTED BY: CEN DESIGNED BY: YLP PROJECT ENGINEER: MAJ *DATE:* **02.15.17** PROJECT NO.: **15065** 

DRAWING: C28 SHEET: 28 OF 31

## PENNY LANE SOUTH



ONE #3 BAR HOOP FOR 6" TWO #3 BAR HOOPS FOR 12"



TYPE 1 CATCH BASIN DETAIL (WSDOT STANDARD PLAN B-5.20-01)

1. SET TO GRADE AND CONSTRUCT ROAD AND GUTTER TO BE FLUSH WITH FRAME.

THROUGH CURB INLET DETAIL (2007 KCRDCS FIG. 7-016 AND FIG. 7-017)

2. SEE SEC. 3.04 FOR JOINT REQUIREMENTS.

- 1. AS AN ACCEPTABLE ALTERNATE TO REBAR, WIRE MESH HAVING A MINIMUM AREA OF 0.12 SQUARE INCHES PER FOOT MAY BE USED. WIRE MESH SHALL
- 2. THE KNOCKOUT DIAMETER SHALL NOT BE GREATER THAN 20". KNOCKOUTS SHALL HAVE A WALL THICKNESS OF 2" MINIMUM TO 2.5" MAXIMUM. PROVIDE A 1.5" MINIMUM GAP BETWEEN THE KNOCKOUT WALL AND THE OUTSIDE OF THE PIPE. AFTER THE PIPE IS INSTALLED, FILL THE GAP WITH JOINT MORTAR IN ACCORDANCE WITH STANDARD SPECIFICATION 9-04.3.
- 3. THE MAXIMUM DEPTH FROM THE FINISHED GRADE TO THE PIPE INVERT SHALL
- 4. FRAME AND GRATE MAY BE INSTALLED WITH FLANGE DOWN OR CAST INTO ADJUSTMENT SECTION.
- 5. THE PRECAST BASE SECTION MAY HAVE A ROUNDED FLOOR AND THE WALLS MAY BE SLOPED AT A RATE OF 1:24 OR STEEPER.
- 6. OPENING SHALL BE MEASURED AT THE TOP OF THE PRECAST BASE SECTION.

PIPE ALLOWANCE	ES
PIPE MATERIAL	MAXIMUM INSIDE DIAMETER
REINFORCED OR PLAIN CONCRETE	12"
ALL METAL PIPE	15"
CPSSP * (STD. SPEC. 9-05.20)	12"
SOLID WALL PVC (STD. SPEC. 9-05.12(1))	15"
PROFILE WALL PVC (STD. SPEC. 9-05.12(2))	15 <b>"</b>

\* CORRUGATED POLYETHYLENE STORM SEWER PIPE

3. PATTERN ON TOP SURFACE OF HOOD SHALL BE 3/16 IN. NON-SKID DIAMOND.

4. BOLT, WASHER, AND NUT SHALL BE GALVANIZED OR CORROSION RESISTANT.

5. SEE SEC. 7.05.

1. CATCH BASINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH ASTM C478 (AASHTO M199) AND ASTM C890 UNLESS OTHERWISE SHOWN ON PLANS OR NOTED IN THE WSDOT/APWA STANDARD SPECIFICATIONS.

2. HANDHOLDS IN ADJUSTMENT SECTION SHALL HAVE 3" MIN. CLEARANCE. STEPS IN CATCH BASIN SHALL HAVE 6" MIN. CLEARANCE. SEE DWG. NO. 2-006, CATCH BASIN DETAILS. HANDHOLDS SHALL BE PLACED IN ALTERNATING GRADE RINGS OR LEVELING BRICK COURSE WITH A MIN. OF ONE HANDHOLD BETWEEN THE LAST STEP AND TOP OF

3. ALL REINFORCED CAST-IN-PLACE CONCRETE SHALL BE CLASS 4000. ALL PRECAST CONCRETE SHALL BE CLASS 4000.

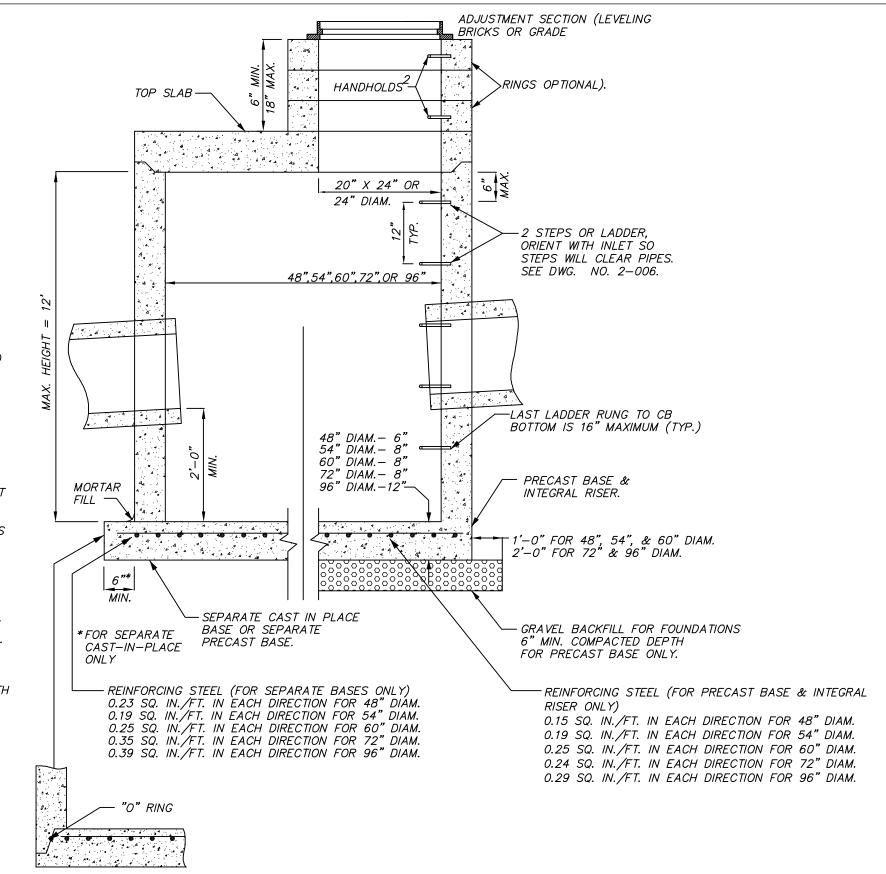
4. PRECAST BASES SHALL BE FURNISHED WITH CUTOUTS OR KNOCKOUTS. KNOCKOUTS SHALL HAVE WALL THICKNESS OF 2" MIN. UNUSED KNOCKOUTS NEED NOT BE GROUTED IF WALL IS LEFT INTACT. PIPES SHALL BE INSTALLED ONLY IN FACTORY KNOCKOUTS UNLESS OTHERWISE APPROVED BY THE ENGINEER.

5. KNOCKOUT OR CUTOUT HOLE SIZE SHALL EQUAL PIPE OUTER DIAM. PLUS CATCH BASIN WALL THICKNESS. MAX. HOLE SIZE SHALL BE 36" FOR 48" CATCH BASIN, 42" FOR 54" C.B., 48" FOR 60" C.B., 60" FOR 72" C.B., 84" FOR 96" C.B. MIN. DISTANCE BETWEEN HOLES SHALL BE 8" FOR 48", 54", AND 60" C.B.; 12" FOR 72" AND 96" C.B.

6. CATCH BASIN FRAMES AND GRATES OR COVERS SHALL BE IN ACCORDANCE WITH SEC. 7.05 AND MEET THE STRENGTH REQUIREMENTS OF FEDERAL SPECIFICATION RR-F-621D. MATING SURFACES SHALL BE FINISHED TO ASSURE NON-ROCKING FIT WITH

- 7. ALL BASE REINFORCING STEEL SHALL HAVE A MIN. YIELD STRENGTH OF 60,000 PSI AND BE PLACED IN THE UPPER HALF OF THE BASE WITH 1" MIN. CLEARANCE.
- 8. MIN. SOIL BEARING VALUE SHALL EQUAL 3,300 POUNDS PER SQUARE FOOT.
- 9. FOR DETAILS SHOWING LADDER, STEPS, HANDRAILS AND TOP SLABS, SEE DWG. NO.

10. SEE THE WSDOT/APWA STANDARD SPECIFICATIONS SEC. 7-05.3 FOR JOINT REQUIREMENTS.



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DRAFTED BY: CEN

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SUBDIVISION City of Sammamish Approval

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NO AS-BUILT INFORMATION ON THIS SHEET.

EXISTING FIELD CONDITIONS AS OF 11/06/19.

for SDP2017-00575 this\_\_\_\_day of\_\_ City Planner Public Works Development Review Engineer

Examined and Approved per SMC 20.05

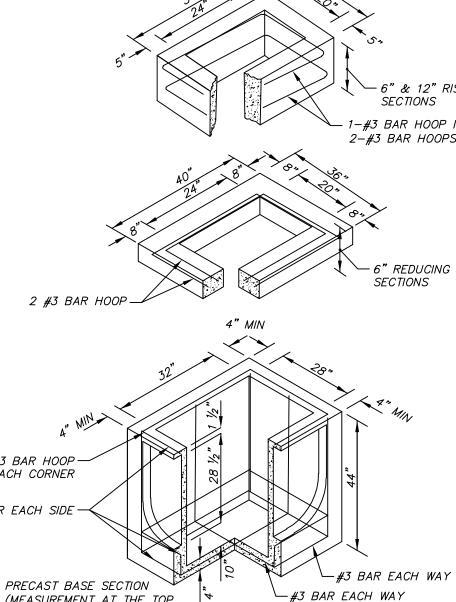
DESIGNED BY: YLP PROJECT ENGINEER: MAJ DATE: **02.15.17** PROJECT NO.: **15065** 

DRAWING: C29 SHEET: **29** OF **31** 

TYPE 2 CATCH BASIN DETAIL (2007 KCRDCS FIG. 7-005)

## DRILL & TAP TWO 5/8"-11NC HOLES - LEVELING PAD 7-1/8" X 3/4" X 2 1/4" THRU FRAME ----SECTION B-B HOOD ATTACHES AS SHOWN. PLANMIN. DRAFT ON THIS SIDE— FACE OF CURB-2 — 1" DIAM. HOLES—/| FOR 3/4" BOLT, WASHER, & NUT, SEE NOTE 4. SECTION C-C SECTION A SECTION A-A 1. MATERIAL IS CAST IRON ASTM A48 CLASS 30. HOOD PLAN DETAIL 2. SEE FIG. 7-018 FOR VANED GRATE. SECTION B

FRAME AND VANED GRATE



**NOTES:** CATCH BASINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH ASTM PLANS OR NOTED IN THE WSDOT/APWA STANDARD SPECIFICATIONS. C478 (AASHTO M 199) & C890 UNLESS OTHERWISE SHOWN ON

2. AS AN ACCEPTABLE ALTERNATIVE TO REBAR, WELDED WIRE FABRIC HAVING A MIN. AREA OF 0.12 SQUARE INCHES PER FOOT MAY BE USED. WELDED WIRE FABRIC SHALL COMPLY TO ASTM A497 (AASHTO M 221). WIRE FABRIC SHALL NOT BE PLACED IN KNOCKOUTS.

ALL REINFORCED CAST-IN-PLACE CONCRETE SHALL BE CLASS 4000.

PRECAST BASES SHALL BE FURNISHED WITH CUTOUTS OR KNOCKOUTS. KNOCKOUTS SHALL HAVE A WALL THICKNESS OF 2" MIN. ALL PIPE SHALL BE INSTALLED IN FACTORY PROVIDED KNOCKOUTS. UNUSED KNOCKOUTS NEED NOT BE GROUTED IF WALL IS LEFT INTACT.

5. KNOCKOUT OR CUTOUT HOLE SIZE IS EQUAL TO PIPE OUTER DIAMETER PLUS CATCH BASIN WALL THICKNESS.

6. KNOCKOUTS MAY BE ON ALL 4 SIDES WITH MAX. DIAM. OF 28". KNOCKOUTS MAY BE EITHER ROUND OR "D" SHAPE.

7. THE TAPER ON THE SIDES OF THE PRECAST BASE SECTION AND RISER SECTION SHALL NOT EXCEED 1/2"/FT. 8. CATCH BASIN FRAME AND GRATE SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS AND MEET THE STRENGTH REQUIREMENTS OF FEDERAL SPECIFICATION RR-F-62ID. MATING SURFACES SHALL BE FINISHED TO ASSURE NON-ROCKING FIT WITH ANY COVER POSITION.

9. FRAME AND GRATE MAY BE INSTALLED WITH FLANGE DOWN OR CAST INTO RISER.

10. MAX. DEPTH FROM FINISHED GRADE TO PIPE INVERT SHALL BE 5'-0". 11. EDGE OF REDUCING SECTION OR BRICK SHALL NOT

BE MORE THAN 2" FROM VERTICAL EDGE OF CATCH BASIN

TYPE 1L CATCH BASIN DETAIL (2007 KCRDCS FIG. 7-004)

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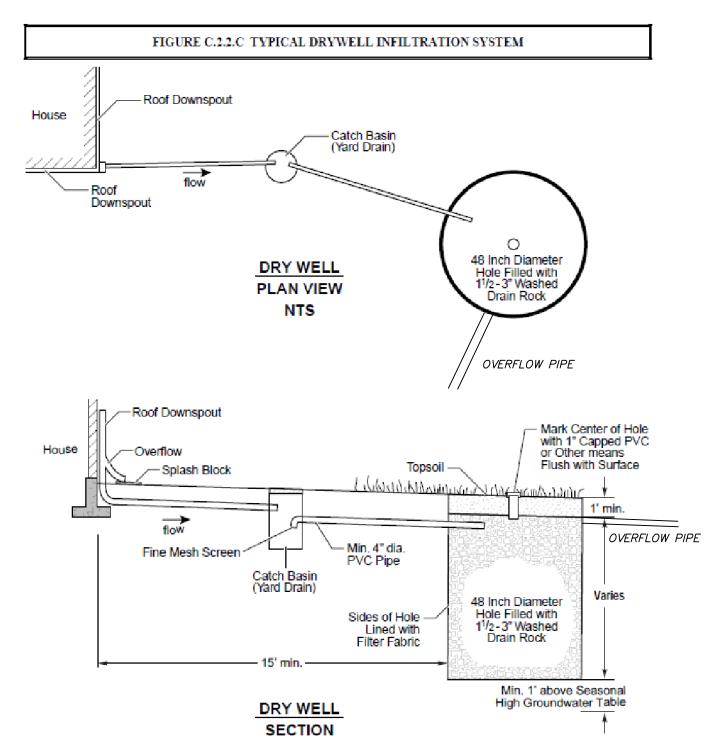
# NOT BE PLACED IN KNOCKOUTS.

-#3 BAR HOOP IN 6" 2-#3 BAR HOOPS IN 12"

#3 BAR HOOP -EACH CORNER #3 BAR EACH SIDE

₩3 BAR EACH WAY (MEASUREMENT AT THE TOP OF THE BASE)

## PENNY LANE SOUTH



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	DETAIL
DRYWELL	DE I AIL

					DRY	WELL	TABLE					
LOT	ROOF AREA (SF)	DRYWELL SIZING (CF/1000 SF)	DRYWELL V REQUIRED	OLUME (CF) PROVIDED	DR YWEL	L DIMENSIC	NS (FT)	FG OVER DRYWELL	TOP OF GRAVEL	BOTTOM OF DRYWELL	INLET ELEVATION	OUTLET ELEVATION
1	1500	230	345.00	360.00	15.00	6.00	4.00	441.10	439.10	434.60	438.10	438.60
2	1500	230	345.00	360.00	15.00	6.00	4.00	443.90	441.90	437.40	440.90	441.40
3	1500	230	345.00	360.00	15.00	6.00	4.00	447.10	445.10	440.60	444.10	444.60
4	1500	230	345.00	360.00	15.00	6.00	4.00	451.10	449.10	444.60	448.10	448.60
5	1500	230	345.00	360.00	15.00	6.00	4.00	453.80	451.80	447.30	450.80	451.30
6	1500	230	345.00	360.00	15.00	6.00	4.00	452.50	450.50	446.00	449.50	450.00
7	1500	230	345.00	360.00	15.00	6.00	4.00	453.00	451.00	446.50	450.00	450.50
8	1500	230	345.00	360.00	15.00	6.00	4.00	453.60	451.60	447.10	450.60	451.10
9	1500	230	345.00	360.00	15.00	6.00	4.00	454.30	452.30	447.80	451.30	451.80
10	1500	230	345.00	360.00	15.00	6.00	4.00	456.30	454.30	449.80	453.30	453.80
11	1500	230	345.00	360.00	15.00	6.00	4.00	458.10	456.10	451.60	455.10	455.60
12	1500	230	345.00	360.00	15.00	6.00	4.00	460.40	458.40	453.90	457.40	457.90
13	1500	230	345.00	360.00	15.00	6.00	4.00	463.10	461.10	456.60	460.10	460.60
14	1500	230	345.00	360.00	15.00	6.00	4.00	475.90	<i>473</i> .90	469.40	472.90	473.40
16	1500	230	345.00	360.00	15.00	6.00	4.00	474.60	472.60	468.10	471.60	472.10
17	1500	230	345.00	360.00	15.00	6.00	4.00	473.60	471.60	<i>467</i> .10	470.60	471.10
18	1500	230	345.00	360.00	15.00	6.00	4.00	472.20	470.20	465.70	469.20	469.70
19	1500	230	345.00	360.00	15.00	6.00	4.00	470.60	468.60	464.10	467.60	468.10
20	1500	230	345.00	360.00	15.00	6.00	4.00	467.00	465.00	460.50	464.00	464.50
22	1500	230	345.00	360.00	15.00	6.00	4.00	477.60	475.60	471.10	474.60	475.10
23	1500	230	345.00	360.00	15.00	6.00	4.00	476.90	474.90	470.40	473.90	474.40

DRYWELL NOTE:

PRIVATE YARD DRAIN WITH SUMP SHALL BE INSTALLED BETWEEN ROOF DRAINS AND DRYWELL PER DETAIL THIS SHEET. CONNECTION BETWEEN YARD DRAIN AND DRYWELL TO BE MADE AT A 2% SLOPE. OVERFLOW OUTLET PIPE SHALL CONNECT TO LOT STORM STUBS OR CATCH BASINS AS SHOWN.

1' - 0 | 1' - 0 END CAP OR PLUG FLOW TO SECOND DISPERSAL TRENCH *IF NECESSARY* MIN 6" PERFORATED NOTCHED GRADE BOARD PIPE LAID FLAT/LEVEL TYPE I CB W/SOLID COVER 2" X 2" NOTCHES 18" O.C. (LOCKING) TYPE I CB W/SOLID COVER INFLUENT PIPE (MAX DESIGN FLOW <0.5 CFS PER TRENCH) -CLEAN OUT WYE FROM PIPE BRANCHING CB'S AS NECESSARY <u>PLAN</u> NTS *GAL VANIZED* **BOLTS** \*20% MAX 2" X 12" > PRESSURE TREATED GRADE 2" GRADE BOARD NOTCHES BOARD . MIN 6" PERFORATED
PIPE LAID FLAT CLEAN\_ (<5% FINES)  $\frac{3}{4}$ " -  $1\frac{1}{2}$ " WASHED  $\frac{3}{2}$ , ROCK THIS TRENCH SHALL BE CONSTRUCTED TO PREVENT POINT DISCHARGE AND OR EROSION. TRENCHES MAY BE PLÁCED NO CLOSER THAN 50 \*15% MAX FOR FLOW FEET TO ONE ANOTHER (100 FEET ALONG FLOWLINE). CONTROL/WATER QUALITY TRENCH AND GRADE BOÀRD MUST BE LEVEL. ALIGN

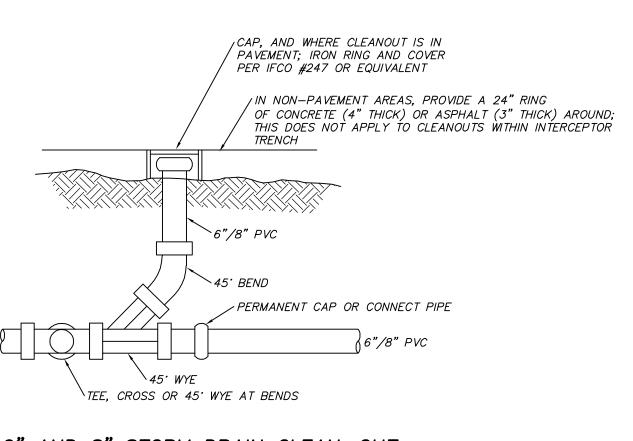
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SUPPORT POST SPACING AS REQUIRED BY SOIL CONDITIONS TO ENSURE GRADE BOARD REMAINS LEVEL.

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TREATMENT IN RURAL

DISPERSAL TRENCH DETAIL



6" AND 8" STORM DRAIN CLEAN-OUT

NTS

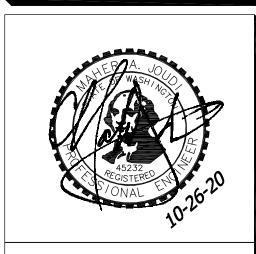
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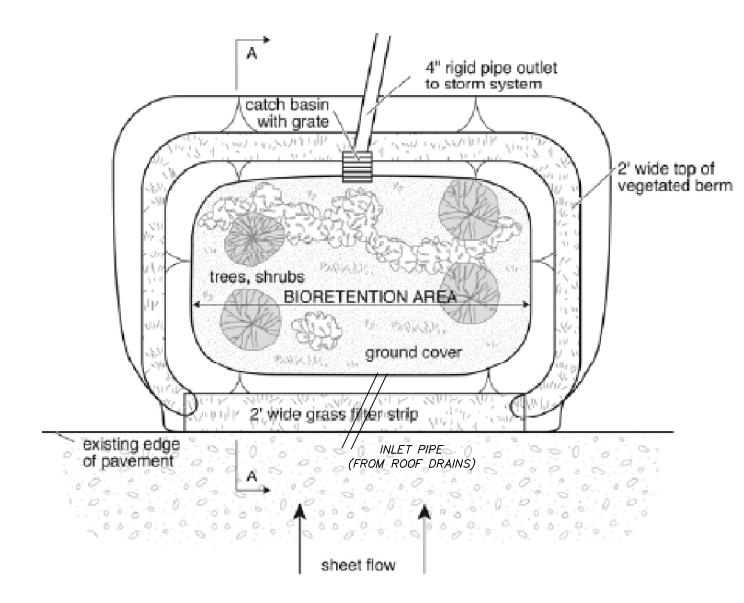
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## PENNY LANE SOUTH

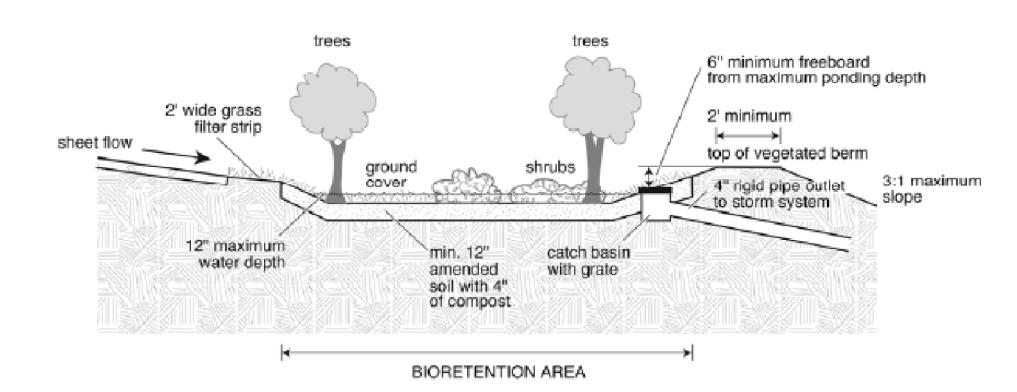
SDP2017-00575

RAIN GARDEN NOTES:

### FIGURE C.2.5.B TYPICAL RAIN GARDEN WITH CONTAINMENT BERM



PLAN VIEW (not to scale)



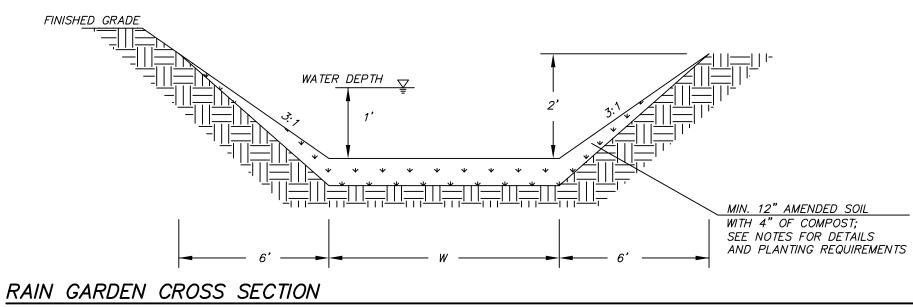
SECTION VIEW A-A (not to scale)

### RAIN GARDEN DETAIL

				R	AIN GA	ARDEN	TABLE				
LOT	ROOF AREA (SF)	STORAGE V REQUIRED	OLUME (CF) PROVIDED	RAIN GAR	DEN DIMENS	SIONS (FT)	TOP OF RAIN GARDEN	BOTTOM OF RAIN GARDEN	INLET ELEVATION	OVERFLOW YD RIM	OUTLET ELEVATION
24	1000	250	250	24	5	1	474.60	472.60	472.60	473.60	472.10
26	2600	650	660	88	4	1	467.50	465.50	465.50	466.50	465.00
27	2050	<i>512.5</i>	520	68	4	1	474.70	472.70	472.70	473.70	472.20
28	2800	700	720	96	4	1	476.75	<i>474.75</i>	474.75	475.75	474.25

### NOTE:

ROOF DRAINS ON LOTS 24 AND 26-28 TO BE CONNECTED TO RAIN GARDENS. FOOTING DRAINS TO BE CONNECTED TO LOT STORM STUBS.



### C.2.5.1 MINIMUM DESIGN REQUIREMENTS

All of the following requirements must be met in order for a rain garden to be applicable to a target impervious surface or new pervious surface:

- A minimum water storage volume equivalent to 3 inches (0.25 feet) of runoff depth from the
  impervious surface area served is required. In other words, the volume in cubic feet shall equal 0.25
  times the square footage of the impervious surface area served (see example calculation in Section
  C.2.5.2 below). For rain gardens serving new pervious surface, a minimum water storage volume
  equivalent to 0.5 inches (0.04 feet) of runoff depth is required.
- 2. The water storage area, containing the minimum required storage volume, shall be 12 inches deep at overflow and have side slopes no steeper than 3 horizontal to 1 vertical. The overflow point of the water storage area shall be at least 6 inches below any adjacent pavement area. The overflow point must be situated so that overflow does not cause erosion damage or unplanned inundation.
- 3. If a containment berm is used to form the water storage area, the berm must be at least 2 feet wide and 6 inches above the 12 inches of water depth. A catch basin or rock pad must be provided to release water when the pond's water level exceeds the 12 inches of water depth. The catch basin may discharge to the local drainage system or other acceptable discharge location via a 4-inch rigid pipe. The rock pad may be used with or without a constructed drainage system downstream. If a rock pad is used, it must be composed of crushed rock, 6-inches deep and 2 feet wide (perpendicular to flow) and must extend at least 4 feet or beyond the containment berm, whichever is greater. The rock pad must be situated so that overflow does not cause erosion damage or unplanned inundation.
- 4. Amended soil consisting of minimum of 4 inches of compost tilled into the upper 12 inches of soil or 12 inches of imported sand/compost blend having 8 to 13% organic material by dry weight is required in the rain garden. Tilling and amending to greater depth is desirable.
- Water tolerant plants such as those in Table C.2.5.A shall be planted in the pond bottom. Plants native to Western Washington are preferred.
- A minimum 5-foot setback shall be maintained between any part of a rain garden and any structure or property line.
- 7. Rain gardens are not allowed in critical area buffers or on slopes steeper than 20%. Rain gardens proposed on slopes steeper than 15% or within 50 feet of a steep slope hazard area or landslide hazard area must be approved by a geotechnical engineer or engineering geologist unless otherwise approved by the DDES staff geologist.
- 8. For *sites* with septic systems, rain gardens must be located downgradient of the primary and reserve drainfield areas. DDES permit review staff can waive this requirement if site topography clearly prohibits subsurface flows from intersecting the drainfield.
- 9. The rain garden must not create flooding or erosion impacts as determined by the DDES. If a rain garden is proposed near a landslide hazard area, erosion hazard area, steep slope hazard area, or a slope steeper than 15%, DDES may require evaluation and approval of the proposal by a geotechnical engineer or engineering geologist.

Common Name	Scientific Name	Spacing (on center)		
Western mannagrass	Glyceria occidentalis	seed		
Vel <b>vetgras</b> s	Holcus mollis	seed		
Shortawn foxtail	Alopecurus aequalis	seed		
Water foxtail	Alopecurus geniculatus	seed		
Spike rush	Eleocharis spp.	4 inches		
Slough sedge	Carex obnupta	6 inches or seed		
Sawbeak sedge	Carex stipata	6 inches		
Sedge	Carex spp.	6 inches		
Slender rush	Juncus tenuis	6 inches		
Water parsley	Oenanthe sarmentosa	6 inches		
Hardstem bulrush	Scirpus acutus	6 inches		
Watercress	Rorippa nasturtium-aquaticum	12 inches		
Small-fruited bulrush	Scirpus microcarpus	12 inches		



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CROWN, LLC

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9675 SE 36TH STREET, SUIT. MERCER ISLAND, WA 980 (206) 588-1147



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Utilities Underground Location Center
(ID,MT,ND,OR,WA)

SUBDIVISION

City of Sammamish Approval
Examined and Approved per SMC 20.05
for SDP2017-00575
this \_\_\_\_day of \_\_\_\_\_, 20\_\_\_\_.

City Planner

Public Works Development Review Engineer

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TE REVISION
13.17 CITY COMMENTS
12.17 CITY COMMENTS
11.19 AS-BUILT
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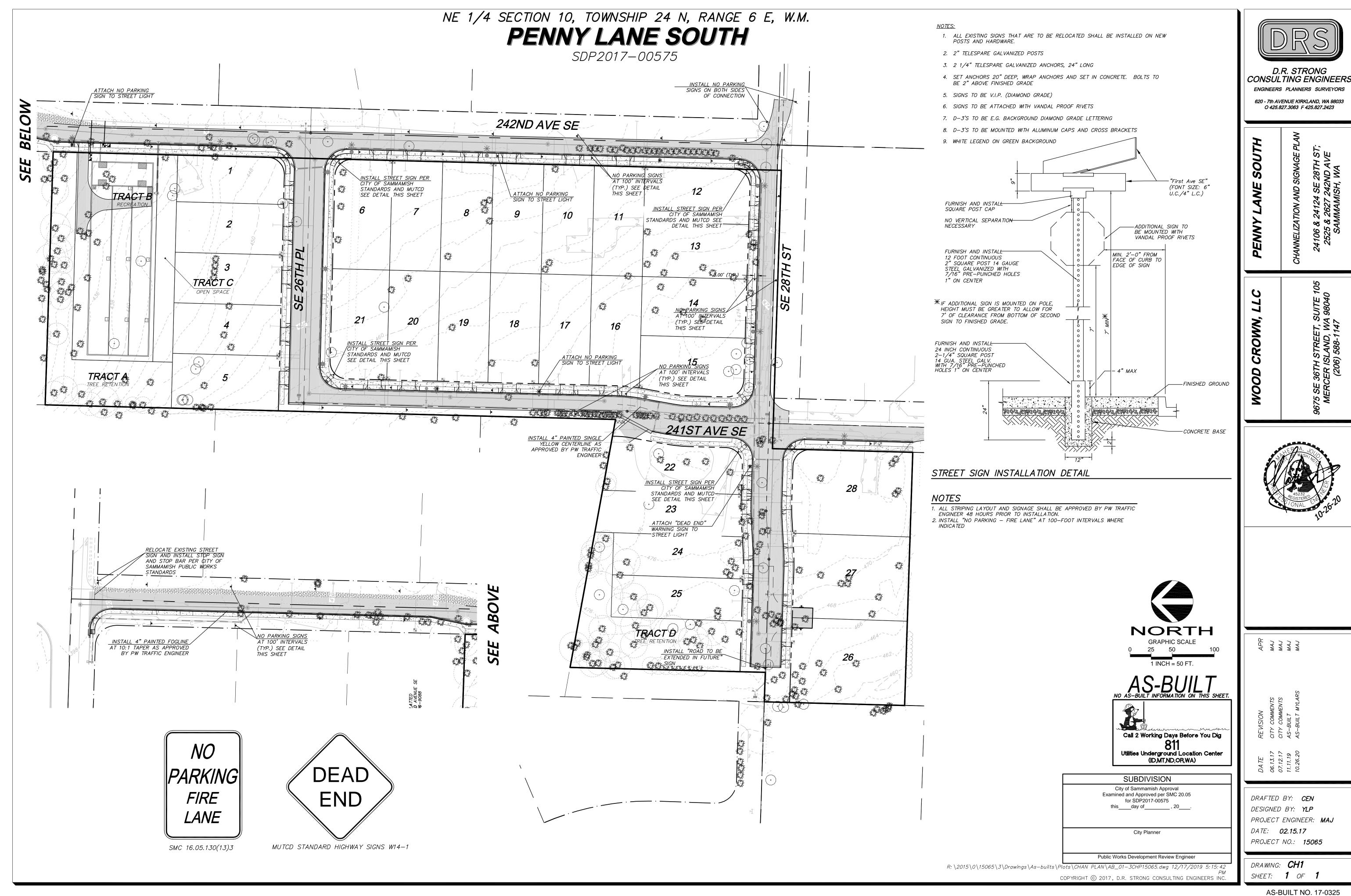
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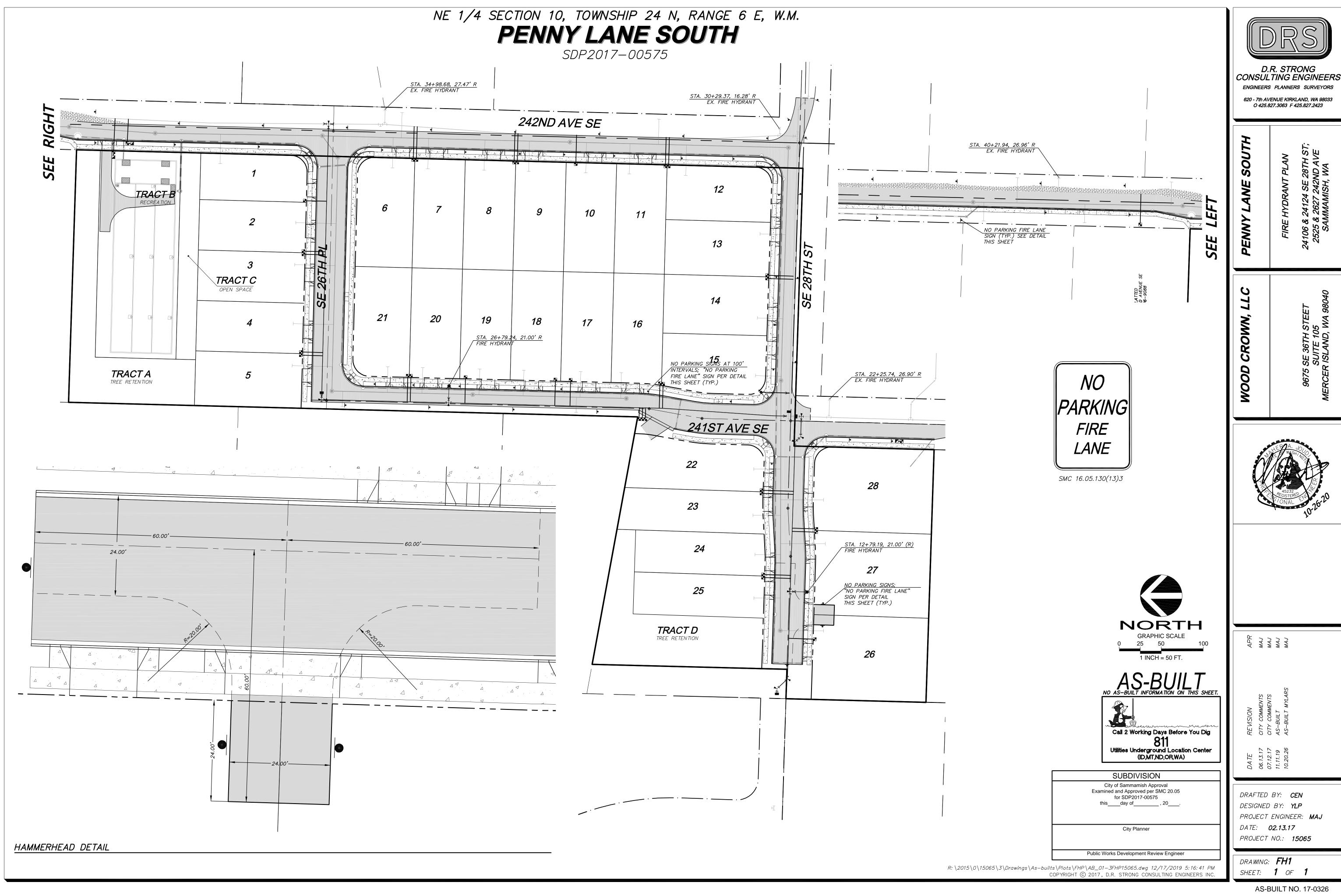
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DATE: 02.15.17

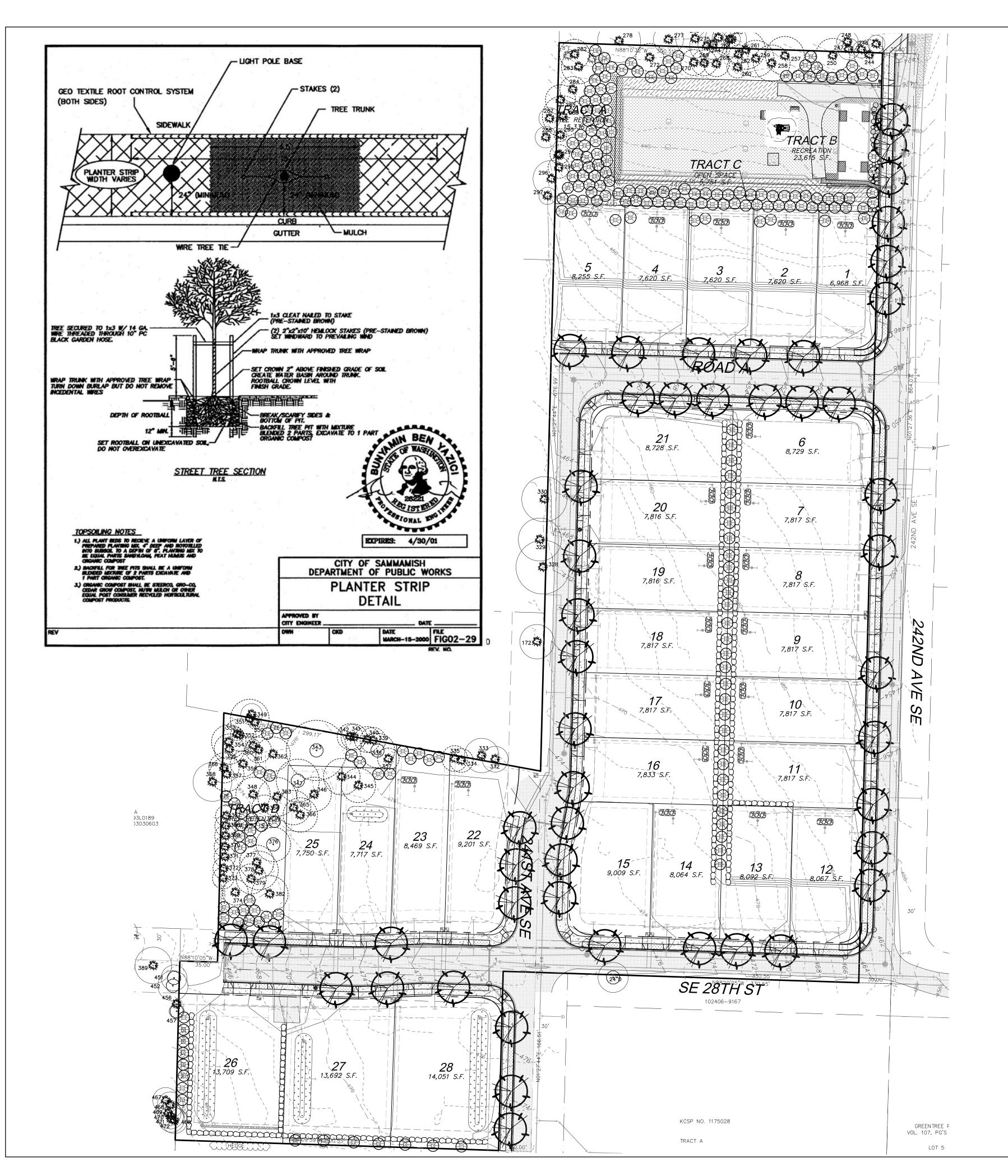
PROJECT NO.: 15065

DRAWING: **C31** SHEET: **31** OF **31** 









## REPLACEMENT TREE MATRIX

229 REPLACEMENT TREES REQUIRED: 210 EVERGREEN TREES, AND 19 DECIDUOUS TREES

STREET TREES BELOW MINIMUM REQUIRED, PER 21A,37,280 (2) (b): 49 STREET TREES x 50% = MAXIMUM 19 DECIDUOUS TREES ALLOWED

OTHER ON-SITE REPLACEMENT TREES, PER PER 21A,37,280 (2) (d) 210 TREES x 100% = 210 1REES

229 REPLACEMENT TREES PROVIDED

## REPLACEMENT TREE PLANT SCHEDULE

Acer rubrum 'Red Sunset' · Pseudotsuaa menziesii Thuja plicata 'Hogan'

Douglas Fir

Western Red Cedar 8' ht min. 92

Full and Matching

Full and Matching Columnar Red Cedar 8' ht min. 38 Full and Matching

Full and Matching, Plant 35' on-center

\*Note: Caliper is measured 6" above finish grade

### LANDSCAPE NOTES

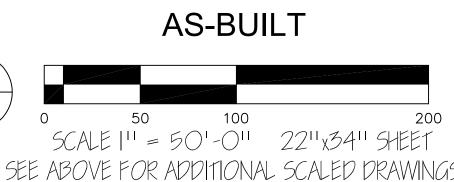
- 1. CONTRACTOR SHALL BE RESPONSIBLE FOR FAMILIARIZING THEMSELVES WITH ALL OTHER SITE IMPROVEMENTS AND CONDITIONS PRIOR TO STARTING LANDSCAPE WORK.
- 2. CONTRACTOR SHALL USE CAUTION WHILE EXCAVATING TO AVOID DISTURBING ANY UTILITIES ENCOUNTERED. CONTRACTOR IS TO PROMPTLY ADVISE OWNER OF ANY DISTURBED UTILITIES. LOCATION SERVICE PHONE 1-800-424-5555

- A. DETERMINATION OF SOIL TEXTURE, INDICATING PERCENTAGE OF ORGANIC MATTER, B. AN APPROXIMATION OF SOIL INFILTRATION RATE, AND
- . MEASURE Ph VALUE. 8. ALL BEDS TO RECEIVE A MINIMUM OF 2" FINE FIR BARK OR SIMILAR APPROVED MULCH
- 9. ALL PLANT MATERIAL SHALL BE FERTILIZED WITH AGRO TRANSPLANT FERTILIZER 4-2-2 PER MANUFACTURER'S SPECIFICATIONS.
- 10. ALL PLANT MATERIAL SHALL CONFORM TO ANLA STANDARDS FOR NURSERY STOCK, LATEST EDITION. ANY REPLACEMENTS MADE AT ONCE.
- A. GENERAL: ALL PLANT MATERIAL FURNISHED SHALL BE HEALTHY REPRESENTATIVES, TYPICAL OF THEIR SPECIES OF VARIETY AND SHALL HAVE A NORMAL GROWTH HABIT. THEY SHALL BE FULL, WELL BRANCHED, WELL PROPORTIONED, AND HAVE A VIGOROUS, WELL DEVELOPED ROOT SYSTEM. ALL PLANTS SHALL BE HARDY UNDER CLIMATIC CONDITIONS SIMILAR TO THOSE IN THE LOCALITY OF THE PROJECT.
- B. TREES, SHRUBS, AND GROUND COVER: QUANTITIES, SPECIES, AND VARIETIES, SIZES AND CONDITIONS AS SHOWN ON THE PLANTING PLAN. PLANTS TO BE HEALTHY, VIGOROUS, WELL FOLIATED WHEN IN LEAF. FREE OF DISEASE, INJURY, INSECTS, DECAY, HARMFUL DEFECTS, AND ALL WEEDS. NO SUBSTITUTIONS SHALL MADE WITHOUT WRITTEN APPROVAL FROM LANDSCAPE ARCHITECT OR OWNER.
- 11. LAWN IS PROPOSED FOR ALL LANDSCAPE STRIPS.

### SOIL AMENDMENT NOTE

AREAS THAT HAVE BEEN CLEARED AND GRADED SHALL HAVE THE SOIL MOISTURE HOLDING CAPACITY RESTORED TO THAT OF THE ORIGINAL UNDISTURBED SOIL NATIVE TO THE SITE TO THE MAXIMUM EXTENT PRACTICABLE. THE SOIL IN ANY AREA THAT HAS BEEN COMPACTED OR THAT HAS SOME OR ALL OF THE DUFF LAYER OR UNDERLYING TOPSOIL DISTURBED SHALL BE AMENDED TO MITIGATE FOR LOST MOISTURE-HOLDING CAPACITY. THE AMENDMENT SHALL TAKE PLACE BETWEEN MAY I AND OCTOBER I. THE TOPSOIL LAYER SHALL BE A MINIMUM OF EIGHT INCHES THICK, UNLESS THE APPLICANT DEMONSTRATES THAT A DIFFERENT THICKNESS WILL PROVIDE CONDITIONS EQUIVALENT TO THE SOIL MOISTURE HOLDING CAPACITY NATIVE TO THE SITE. THE TOPSOIL LAYER SHALL HAVE AN ORGANIC MATTER CONTENT OF BETWEEN FIVE TO TEN PERCENT DRY WEIGHT AND A PH SUITABLE FOR THE LANDSCAPE PLANTS. WHEN FEASIBLE, SUBSOILS BELOW THE TOPSOIL LAYER SHOULD BE SCARIFIED AT LEAST FOUR INCHES WITH SOME INCORPORATION OF THE UPPER MATERIAL TO AVOID STRATIFIED LAYERS. COMPOST USED TO ACHIEVE THE REQUIRED SOIL ORGANIC MATTER CONTENT MUST MEET THE DEFINITION OF "COMPOSTED MATERIALS" IN WAC 173-350-220.

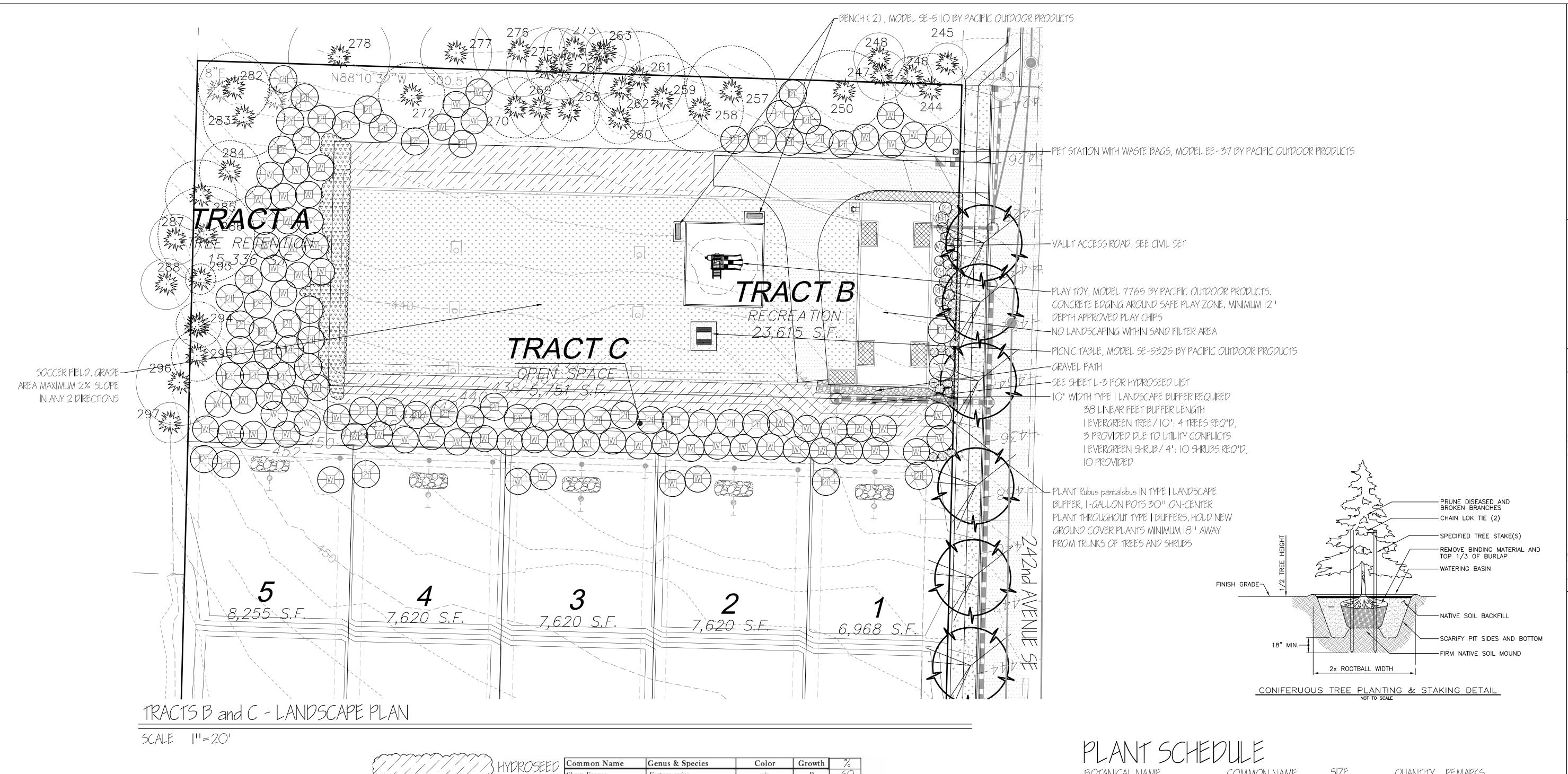
THIS REQUIREMENT DOES NOT APPLY TO AREAS OF THE SITE COVERED BY IMPERVIOUS SURFACE, INCORPORATED INTO A DRAINAGE FACILITY OR ENGINEERED AS A STRUCTURAL FILL OR SLOPE.

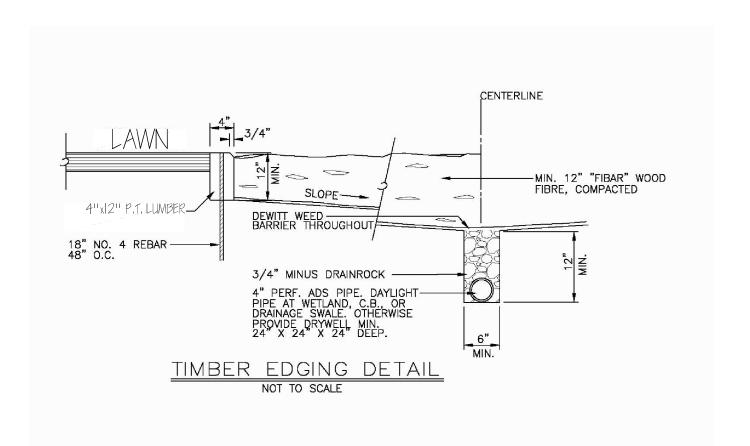


STATE OF

JOB NUMBER: DRAWING NAME: ESIGNER: DRAFTING BY:

10.24.15 SCALE: AS SHOWN JURISDICTION: SAMMAMISH





### Blue Wildrye Elymus glaucus n/a Bromus marginatus Aountain Brome n/a Deschampsia caespitosa **Fufied Hairgrass** n/a Linum perenne lewisii Blue Blue Flax Walflower Cheiranthus cheiri Achillea millefolium Western Yarrow White Rudbeckia hirta Black-eyed Susan Yellow/Black

### DESCRIPTION

Western Native is a unique blend of native grasses and wildflowers formulated to beautify and naturalize your landscape by providing seasonal color, enhanced wildlife habitat, and natural soil stabilization. Western Native will provide a low maintenance, relatively inexpensive, long lasting, and ever-changing addition to any landscape or environment.

Lupinus perennis

Blue/Purple

Western Native is made up of 92% grasses and 8% wildflowers native to Western Washington, and is suited to any planting site west of the Cascade Mountains less than 3000 ft. eleva-

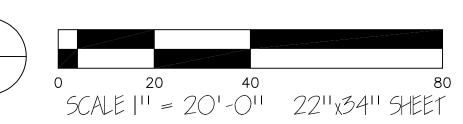
<b>ESTABLISHMENT</b>
& MANAGEMENT

Seeding rate: 2.25 lbs. per 1000 sq. ft. 100 lbs. per acre

	BOTANICAL NAME	COMMON NAME	71ZC	QUANTITY	REMARKS
	SHRUBS and GROUND COVER				
	Lavandula sp. 'Hidcote'	Lavender	I-gallon	7	Full and Matching
(RH)	Rhododendron	Rhodie	1811 ht	5	Full and Matching
	Miscanthus sinensis	Maiden Grass	18" ht	6	Full and Matching
EH	Vaccinium ovatum	Evergreen Huckleberry	min. 2411 ht	4	Full and Matching
00	Mahonia aquifolium	Tall Oregon Grape	min, 24" ht		Full and Matching
	Erica sp. 'Med. White'	Heather	l gallon	As req'd	Plant 24" on-center
* * * * * * * * * * * * * * * * * * *	LAWN				

2"-3" depth arborist wood chip mulch

## **AS-BUILT**

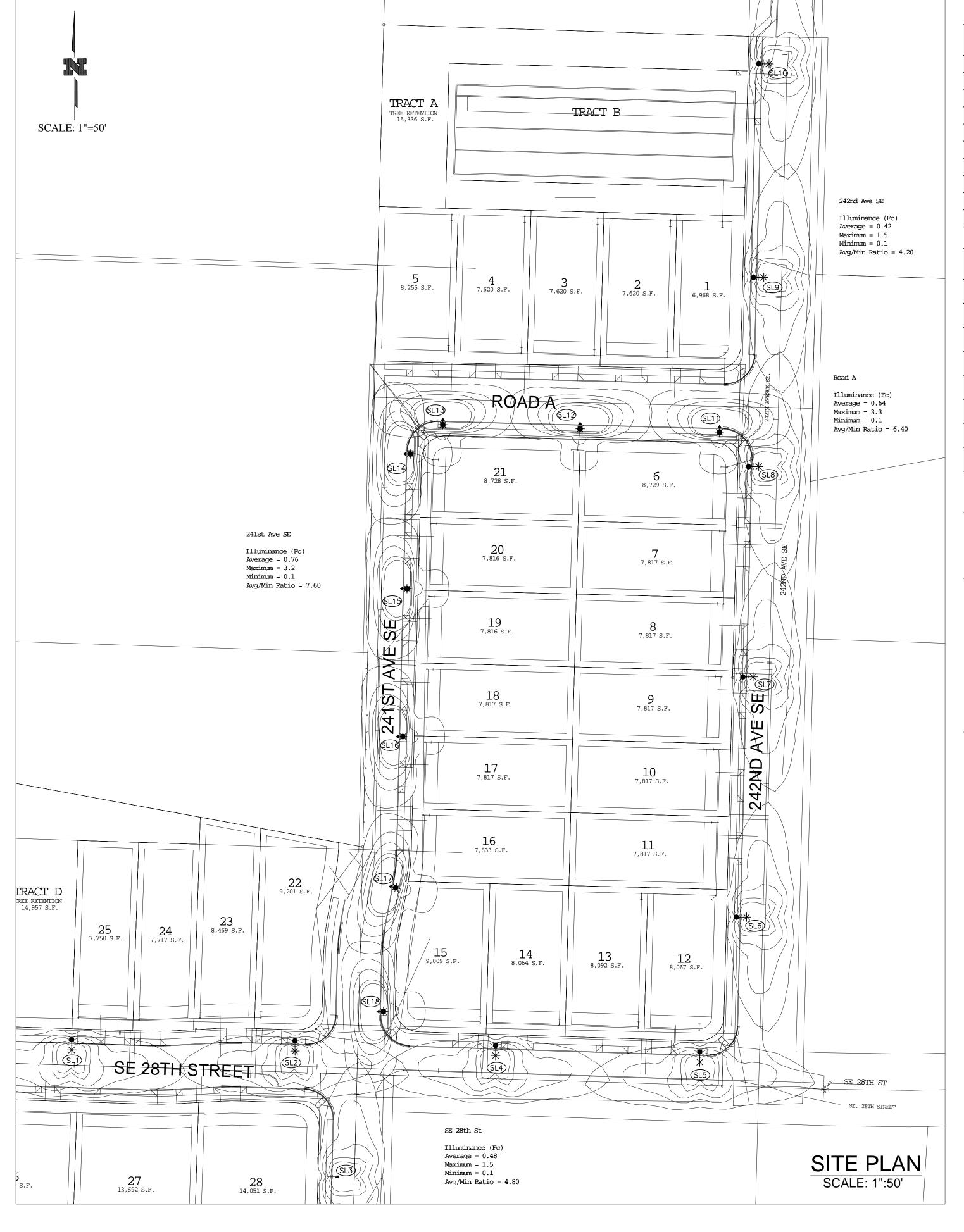


AS-BUILT NO. 17-0328

STATE OF WASHINGTON

JOB NUMBER: DRAWING NAME: ESIGNER: DRAFTING BY: SCALE: JURISDICTION: SAMMAMISH

10.24.15 AS SHOWN



			POLE		L	LUMINAIRE TUBE			wo#	BILLING	TOTAL	ĺ	
SITE #	GRID#	INTOLIGHT TAG #	TYPE	мтс нт.	ARM	WATTS	STYLE	TUBE LENGTH	TUBE DIAMETER	(INTOLIGHT)	SCH.	CONN LOAD	NOTES
SL1			GREY OCT CONC	25'	8'	53W	CHFL LED (FULL CUTOFF)	5'	18"	105084108	51	120/240	
SL2			GREY OCT CONC	25'	8'	53W	CHFL LED (FULL CUTOFF)	5'	18"	105084108	51	120/240	
SL3			GREY OCT CONC	25'	8'	53W	CHFL LED (FULL CUTOFF)	5'	18"	105084108	51	120/240	
SL4			GREY OCT CONC	25'	8'	53W	CHFL LED (FULL CUTOFF)	5'	18"	105084108	51	120/240	
SL5			GREY OCT CONC	25'	8'	53W	CHFL LED (FULL CUTOFF)	5'	18"	105084108	51	120/240	
SL6			GREY OCT CONC	25'	8'	53W	CHFL LED (FULL CUTOFF)	5'	18"	105084108	51	120/240	
SL7			GREY OCT CONC	25'	8'	53W	CHFL LED (FULL CUTOFF)	5'	18"	105084108	51	120/240	
SL8			GREY OCT CONC	25'	8'	53W	CHFL LED (FULL CUTOFF)	5'	18"	105084108	51	120/240	
SL9			GREY OCT CONC	25'	8'	53W	CHFL LED (FULL CUTOFF)	5'	18"	105084108	51	120/240	
SL10			GREY OCT CONC	25'	8'	53W	CHFL LED (FULL CUTOFF)	5'	18"	105084108	51	120/240	

	POLE			LUMINAIRE		TUBE		wo#	BILLING	TOTAL			
SITE #	GRID#	INTOLIGHT TAG#	TYPE	мтс нт.	ARM	WATTS	STYLE	TUBE LENGTH	TUBE DIAMETER	(INTOLIGHT)	SCH.	CONN LOAD	NOTES
SL11			OCT CONC BLK	15'	N/A	75W	3000K TYPE III KING K308 LED (FULL CUTOFF)	4'	18"	105084108	51	120/240	
SL12			OCT CONC BLK	15'	N/A	75W	3000K TYPE III KING K308 LED (FULL CUTOFF)	4'	18"	105084108	51	120/240	
SL13			OCT CONC BLK	15'	N/A	75W	3000K TYPE III KING K308 LED (FULL CUTOFF)	4'	18"	105084108	51	120/240	
SL14			OCT CONC BLK	15'	N/A	75W	3000K TYPE III KING K308 LED (FULL CUTOFF)	4'	18"	105084108	51	120/240	
SL15			OCT CONC BLK	15'	N/A	75W	3000K TYPE III KING K308 LED (FULL CUTOFF)	4'	18"	105084108	51	120/240	
SL16			OCT CONC BLK	15'	N/A	75W	3000K TYPE III KING K308 LED (FULL CUTOFF)	4'	18"	105084108	51	120/240	
SL17			OCT CONC BLK	15'	N/A	75W	3000K TYPE III KING K308 LED (FULL CUTOFF)	4'	18"	105084108	51	120/240	
SL18			OCT CONC BLK	15'	N/A	75W	3000K TYPE III KING K308 LED	4'	18"	105084108	51	120/240	

### SCOPE OF PROJECT:

AT SITES SL1, SL2, SL3, SL4, SL5, SL6, SL7, SL8, SL9, & SL10:

-INSTALL NEW 25'MH GREY OCT CONC POLE -INSTALL NEW 53W-CHFL LED (FULL CUTOFF) LUMINAIRE W/8' ARM ON NEW POLE

AT SITES SL11, SL12, SL13, SL14, SL15, SL16, SL17, & SL18:

- -INSTALL NEW 15'MH BLACK CONCRETE POLE
- -INSTALL NEW 75W 3000K TYPE III KING K308 LED (FULL CUTOFF) LUMINAIRE ON NEW POLE

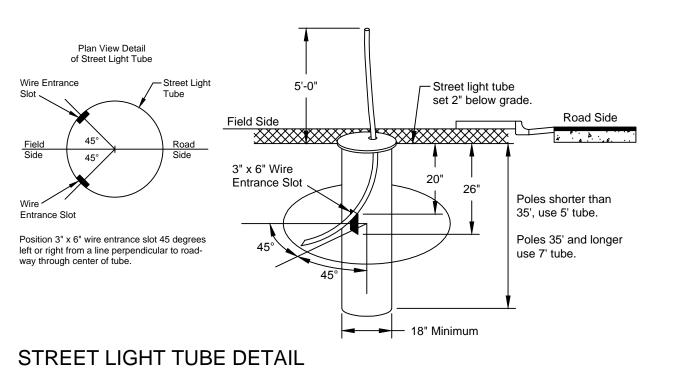
### INTOLIGHT STREET LIGHT NOTES

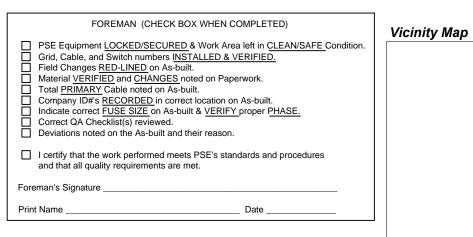
### POTELCO:

- 1. ALL STREET LIGHTING POLES ARE TO BE INSTALLED PER STANDARD 6375.4800 (page #2) IN THE "LINE WORK PRACTICES
- 2. ALL POLES (WOOD, CONCRETE OR FIBERGLASS) ARE TO BE SET PLUMB AND EMBEDDED TO THE GROUND LINE MARKED ON THE
- 3. BACKFILL AROUND POLE WITH 5/8" MINUS GRAVEL AND COMPACT IN 6" LIFTS. (PEA GRAVEL AND NATIVE SOILS ARE NOT ACCEPTABLE.) APPROXIMATELY 1 CU. YD. OF 5/8" MINUS CRUSHED ROCK WILL BE REQUIRED.
- 4. IN ALL SHOEBOX AND COBRAHEAD INSTALLATIONS, THE LUMINAIRE MUST BE LEVELED.

### DEVELOPER/CUSTOMER:

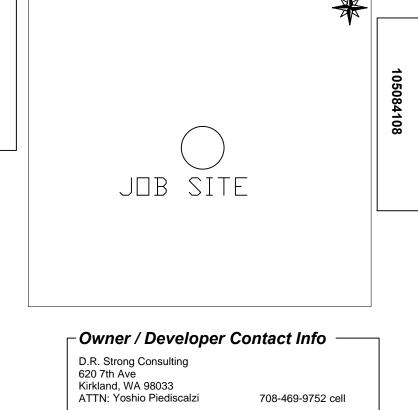
- 1. THE DEVELOPER IS REQUIRED TO SUPPLY AND INSTALL PLASTIC (NON PAPER) STREET LIGHT TUBES (MINIMUM 18" DIAMETER) TO AID IN THE INSTALLATION OF THE STREET LIGHTING POLES.
- 2. DEVELOPER MUST SUPPLY DURABLE LID/COVER AT EACH STREET LIGHT TUBE.





PROJE	CT PHASE	NOTIF#	ORDER#
PWR_	Superior		
	St Light		105084108
	Installation		
	Removal		
	Temporary		
	Job Order		
GAS	Distribution		
	HP Main		
	HP Svc/MS		
CABLE	E TV		
PHON	E		
Project	t Manager Conta	ct Information:	
	RAWLEY ROBI 206-604-31	NS 51 <i>Cell Phone</i>	

<u>Developer</u> Yes



For contacts below dial 1-88	8-CALL PSE (225-5773)
	CALL (800) 424-5555
	2 BUSINESS DAYS BEFORE YOU DIG

105084108

	THIS SKETCH NOT TO BE RELIED UPON FOR EXACT LOCATION OF EXISTING FACILITIES											
V	X NEW BUSINESS			CORRECTIVE /		REAL E	REAL ESTATE/EASEMENT			PERMIT		
NEW BOSINESS			10 DAY WAIVED				N/A			N/A		
3							FUNCT	ION	CONTACT	PHONE NO	DAT	
2						ACCOL	JNT MGR	Brynja Almazan	425-456-383	33		
1								POWER	Kayla King	425-577-239	92	
REV#		BY	DES	DESCRIPTION			ENGR -	- GAS				
COUNTY		Emer Sect	Gas	Wk Ctr	POWER	WK CTR	DRAW	N BY	Kayla King	425-577-239	92 2/10/	
KING	G					3515		ED BY				
1/4 SEC		OP MAP			PLAT M	LAT MAP		VED BY				
N/A	١						FOREMAN #1					
U-MAP NO (F	POWER)	ОН СКТ МА	۱P	UG CKT	MAP	CIRCUIT NO	FOREMAN #2					
2406E	037	2406E04	10	2406	E037	PIN - 27	MAPPII	NG				
					JOIN	T FACILITIES A	RRANGE	MENTS				
	UTILITIES		N/A			N/A			N/A	N'	I/A	
(	CONTACT N/A N/A						N/A	N'	I/A			
	PHONE# N/A N/A N/A N/A											
	DI IC	CT .					00			INCIDENT	MAOP	
ADGT.	PUG				PEN	NY LANE	SOU	TH		Coo Ordor	Float Ord	
	Gas Order Elect Order											

**AS-BUILT** 

PSE SOUND ENERGY

NEW INSTALLATION SE 24TH ST, SAMMAMISH, WA 98074 A\$-BUILETONO. 17-00329

### **DESIGN NOTES:**

REFERENCE: 1. DR. Strong Consulting Engineers, Inc., Preliminary Grading Plan, undated 2. Earth Solutions NW, LLC, Geotechnical Engineering Study, Project No. ES-4006, dated September 17, 2015

### The following assumptions were used:

Internal angle of friction for reinforced soil = 32 degrees (design only) Unit weight of reinforced soil = 125 pcf

Maximum wall height = 12 feet

Batter of wall = Vertical or 1H : 6V Surcharges (Where Applicable) = 2500 plf Footing Strip Load

Rockery construction is a craft. The skill and experience of the builder will largely dictate the success of the construction.

A rockery is a protective system with respect to the weathering and erosion process on an exposed soil face.

Maximum inclination of the slopes above and in front of rockeries should be 2:1 (horizontal: vertical).

### Minimum thickness of rock filter layer = 18 inches.

Rockeries greater than 6 feet in height should be installed under the observation of the Geotechnical Engineer.

The long dimension of the rocks should extend back towards the cut or fill face to provide maximum stability.

Rocks should be placed to avoid continuous joint planes in vertical or lateral directions. Each rock should bear on two or more rocks below it, with good flat-to-flat contact.

Rock designations and approximate weights are provided below.

For Fill Rockeries, it is imperative that Structural Fill compaction extend all the way to the back of the Rockery and Filter Drain Rock Zone. Reduced lift thickness and light compaction equipment may be required to fully achieve the required compaction.

Size	Approximate Weight - lbs.	Approximate Diameter
1 Man	50 - 200	12" - 18"
2 Man	200 - 700	18" - 28"
3 Man	700 - 2000	28" - 36"
4 Man	2000 - 4000	36" - 48"
5 Man	4000 - 6000	48" - 54"
6 Man	6000 - 8000	54" - 60"

### **GEOGRID SOIL REINFORCEMENT**

- A. Geosynthetic reinforcement shall consist of geogrids manufactured specifically for soil reinforcement applications and shall be manufactured from high tenacity polyester yarn or high density polyethylene. Polyester geogrid shall be knitted from high tenacity polyester filament yarn with a molecular weight exceeding 25,000 Meg/m and a carboxyl end group values less than 300. Polyester geogrid shall be coated with an impregnated PVC coating that resists peeling, cracking and stripping.
- Geogrid shall consist of Miragrid (See Geogrid Schedule).
- Manufacturing Quality Control:
- The geogrid manufacturer shall have a manufacturing quality control program that includes QC testing by an independent laboratory.
- The QC testing shall include:
  - ..Tensile Strength Testing ..Melt Flow Index (HDPE)
  - ..Molecular Weight (Polyester)

### STRUCTURAL FILL

Structural Fill shall consist of granular well graded material with a fines content of less than 25 percent (percent passing the #200 sieve based on the minus three-quarters inch fraction). Some rockery applications may require the use of "select" free draining Structural Fill Material. Structural Fill Material shall be approved and tested by the Geotechncial Engineer.

### STRUCTURAL GEOGRID INSTALLATION

- A. Geogrid shall be oriented with the highest strength axis perpendicular to the rockery
- Geogrid Reinforcement shall be placed at the strengths, lengths and elevations shown on the construction design drawings or as directed by the Engineer.
- The geogrid shall be laid horizontally on compacted backfill and extend to the back of the rockery. The geogrid shall be pulled taut, and anchored prior to backfill placement on the geogrid.
- Geogrid Reinforcements shall be continuous throughout their embedment lengths and placed side-by-side to provide 100% coverage at each level. Spliced connections between shorter pieces of geogrid or gaps between adjacent pieces of geogrids are not permitted.

### REINFORCED BACKFILL PLACEMENT

- Reinforced Backfill shall be placed, spread and compacted in such a manner that minimizes the development of slack in the geogrid and installation damage.
- Reinforced Backfill shall be placed and compacted in lifts not to exceed 6 inches where hand compaction is used,12 inches where heavy compaction equipment is used. Lift thickness shall be decreased to achieve the required density as required.
- Reinforced Backfill shall be compacted to 95% of the maximum density as determined by ASTM D-1557-91. The moisture content of the backfill material prior to and during compaction shall be at or near the optimum moisture content.
- The required compaction shall extend all the way to the back of the Rockery and Filter Drain Rock Zone.

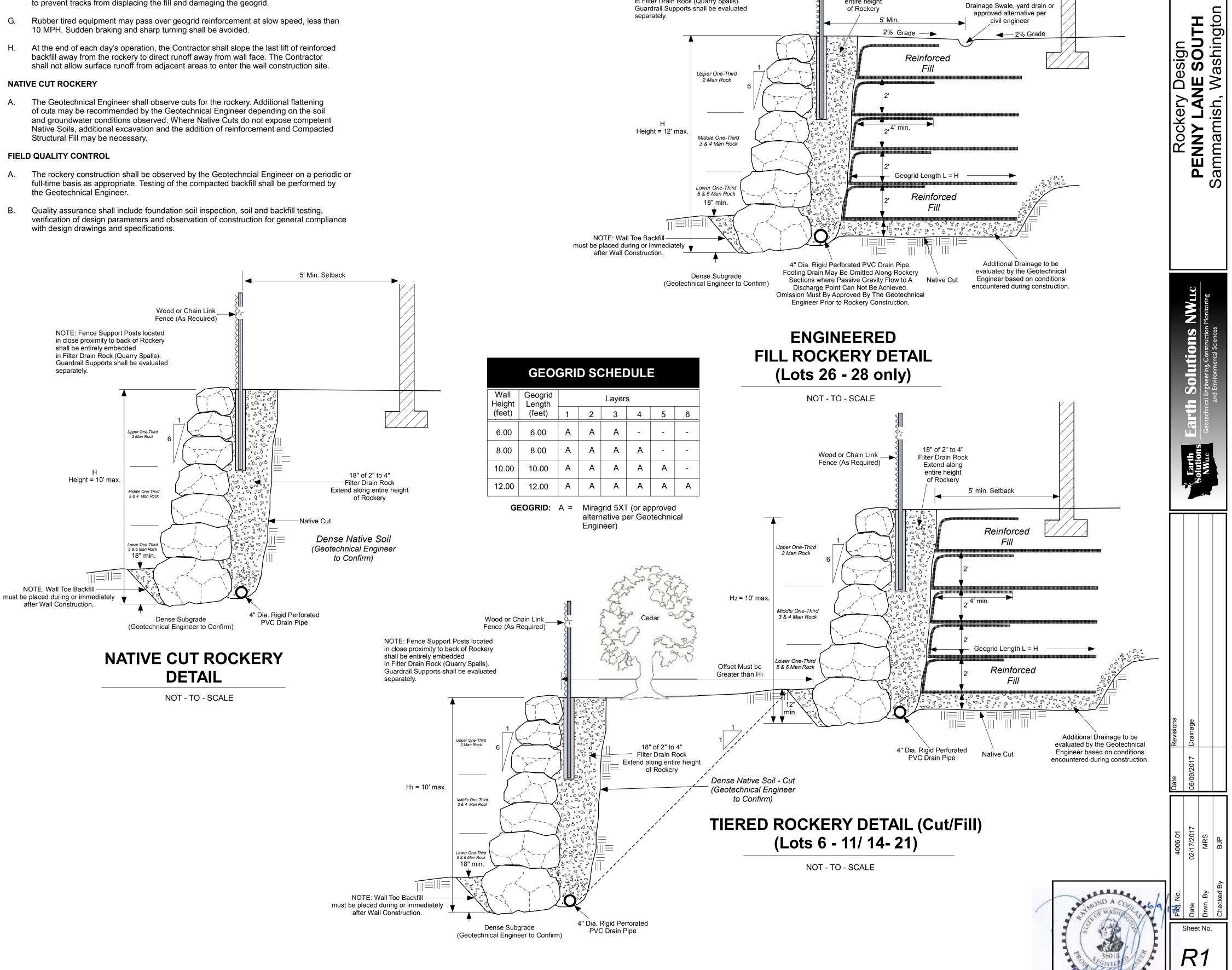
- Only lightweight hand-operated equipemnt shall be allowed within 3 feet of the back of the rockery.
- Tracked construction equipment shall not be operated directly upon the geogrid reinforcement. A minimum fill thickness of 6 inches is required prior to operation of tracked vehicles over the geogrid. Tracked vehicle turning should be kept to a minimum to prevent tracks from displacing the fill and damaging the geogrid.
- Rubber tired equipment may pass over geogrid reinforcement at slow speed, less than 10 MPH. Sudden braking and sharp turning shall be avoided.
- H. At the end of each day's operation, the Contractor shall slope the last lift of reinforced backfill away from the rockery to direct runoff away from wall face. The Contractor shall not allow surface runoff from adjacent areas to enter the wall construction site.

### **NATIVE CUT ROCKERY**

The Geotechnical Engineer shall observe cuts for the rockery. Additional flattening

### FIELD QUALITY CONTROL

- A. The rockery construction shall be observed by the Geotechnoial Engineer on a periodic or full-time basis as appropriate. Testing of the compacted backfill shall be performed by
- Quality assurance shall include foundation soil inspection, soil and backfill testing,



10' Min. Setback

Drainage Swale, yard drain or

approved alternative per

civil engineer

**AS-BUILT** 

PARADA

→ 2% Grade

18" of 2" to 4"

Filter Drain Rock

Extend along

entire height

of Rockery

2% Grade —

Reinforced

Wood or Chain Link

Fence (As Required)

NOTE: Fence Support Posts located

in close proximity to back of rockery

in Filter Drain Rock (Quarry Spalls).

Guardrail Supports shall be evaluated

shall be entirely embedded